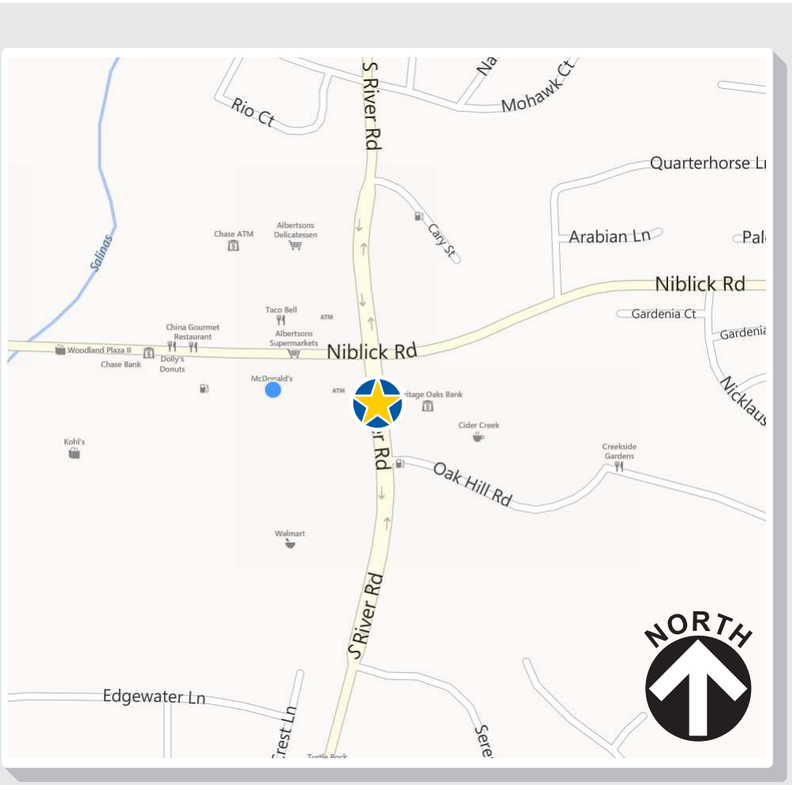
DRC Item 2

McDonald's

• McDonald's • 186 Niblick Rd. • Paso Robles, CA



1 VICINITY MAP

SCALE: NTS

superior electrical advertising

1700 West Anaheim Street
Long Beach, California
90813-1195
Phone: 562.495.3808
Facsimile: 562.435.1867

www.superiorsigns.com

Project:
McDonald's

Address:
186 Niblick Rd.,
Paso Robles, CA

Account Manager:
Chris Janocha

Job No.: **250171-02**

Revision History:
R1 1/28/25 LR New Drawing
R2 5/22/25 LR Order engineering PO 11956

Notes: The colors depicted here are a graphic representation and vary based on monitor or printer calibration. See color specifications.

ELECTRIC SIGN

This sign is intended to be installed in accordance with the requirements of Article 600 of the National Electrical Code and/or other applicable local codes. This includes proper grounding and bonding of the sign.

• CONSTRUCTION APPROVALS •

Acct. Mgr: Date:

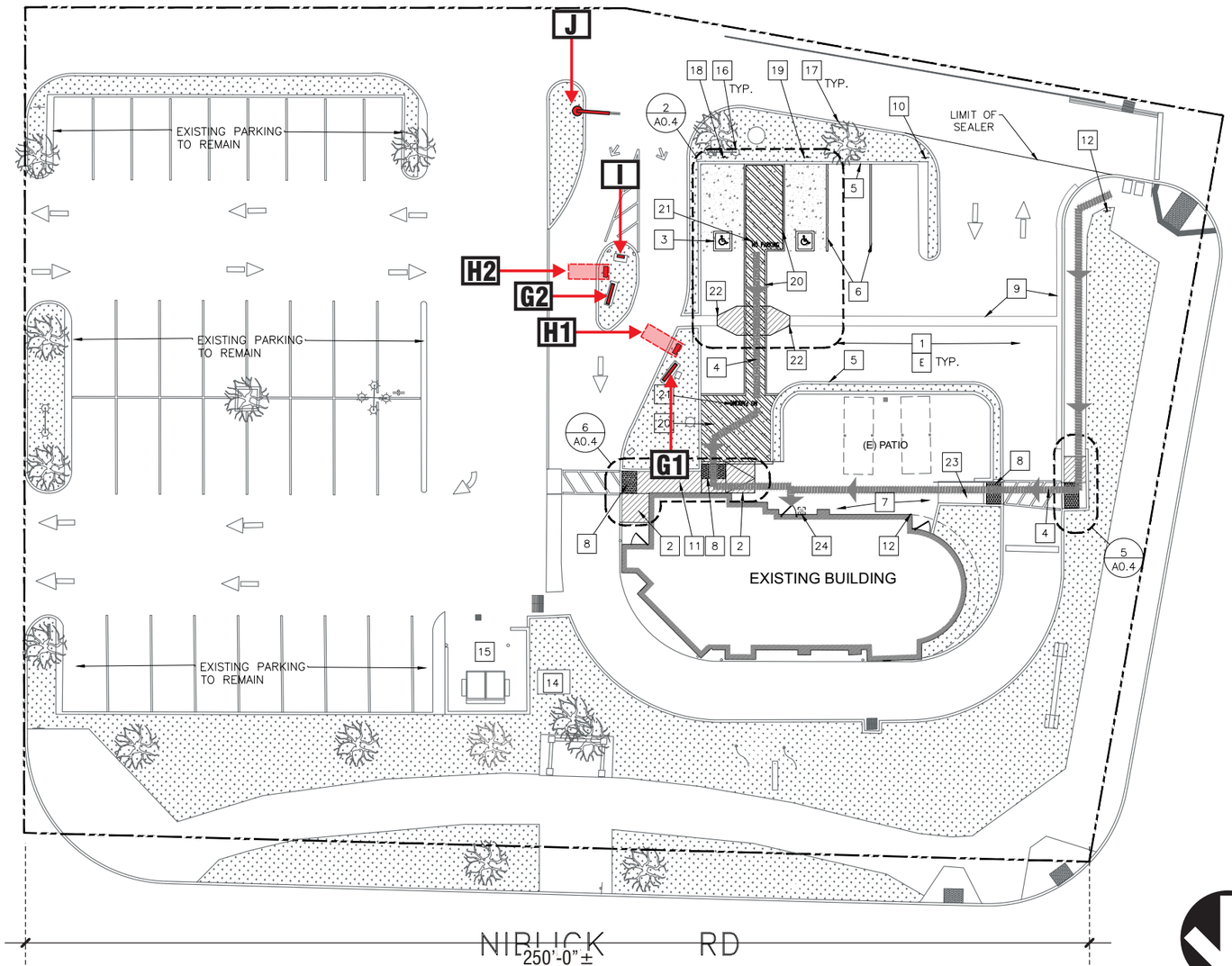
Design: Date:

Mfg/QC: Date:

S-01

DRC Item 2

SIGN SCHEDULE - McDONALD'S SIGNAGE					
NO.	DESCRIPTION	ILLUM.	AREA	QUANT.	TOTAL
G	OUTDOOR MENUBOARD	Y	19.8	2	39.6
H	SLIM COD CANOPY	Y	1.49	2	2.98
I	OUTDOOR PRESELL	Y	9.9	1	9.9
J	GATEWAY CLEARANCE	N	5.0	1	5.0
L					
	TOTAL SQ. FOOTAGE =				57.48



1 **SITE PLAN**

APPROXIMATE SCALE: 1/32" = 1'-0"

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R1 1/28/25 LR New Drawing
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• CONSTRUCTION APPROVALS •

Acct. Mgr: Date:

Design: Date:

Mfg/QC: Date:

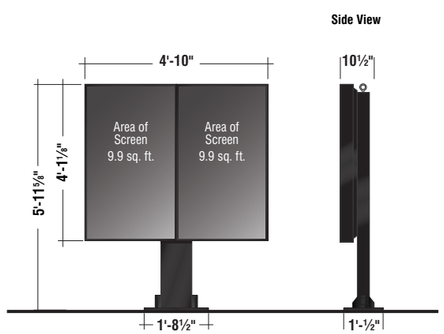
S-02

DRC Item 2

** ALL FOOTINGS BY THE GENERAL CONTRACTOR **

(for signs on this page)

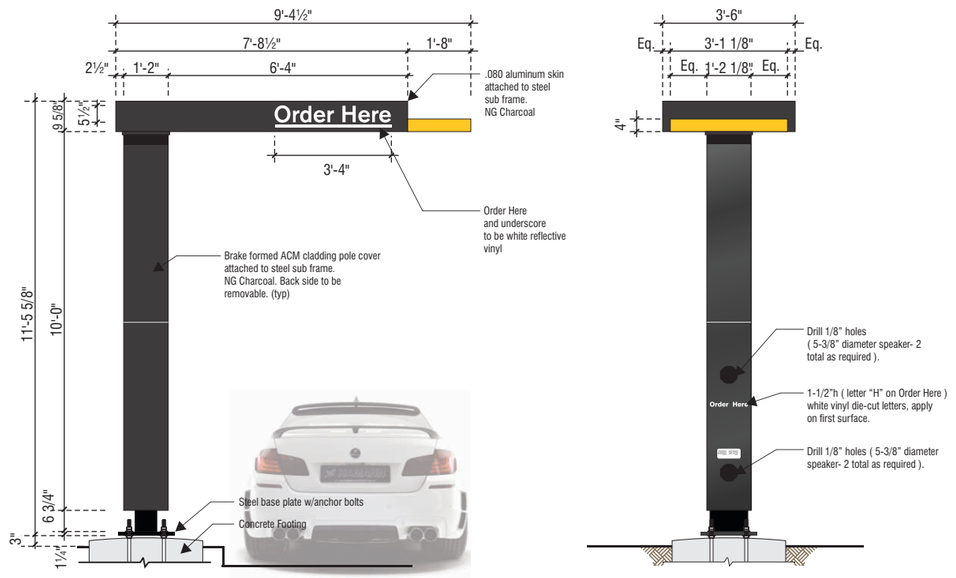
G1 G2



OUTDOOR MENUBOARD — 19.8 Sq. Ft.

Two (2) Units Required — Scale: 1/4" = 1'-0"

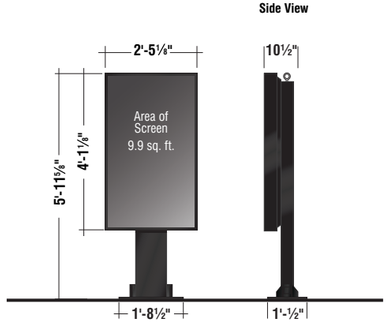
H1 H2



SLIM C.O.D. "SPRINGBOARD" CANOPY — 1.49 Sq. Ft

Two (2) Units Required — Scale: 1/4" = 1'-0"

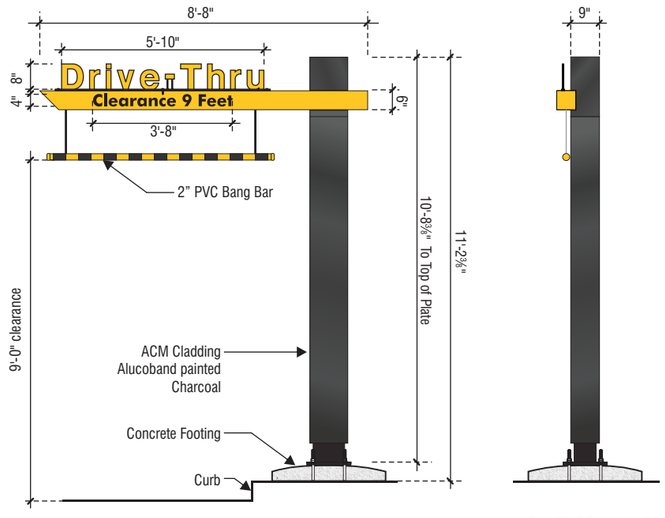
I



OUTDOOR PRESELL — 9.9 Sq. Ft.

One (1) Unit Required — Scale: 1/4" = 1'-0"

J



GATEWAY CLEARANCE SIGN — 1.2 Sq. Ft. /3.8 Sq. Ft. = 5 Sq. Ft

One (1) Unit Required — Scale: 1/4" = 1'-0"

superior electrical advertising

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90813-1195
Phone: 562.495.3808
Facsimile: 562.435.1867

www.superiorsigns.com

Project:
McDonald's

Address:
186 Niblick Rd.,
Paso Robles, CA

Account Manager:
Chris Janocha

Job No.: **250171-02**

Revision History:
R1 1/28/25 LR New Drawing
R2 5/22/25 LR Order engineering
PO 11956

Notes: The colors depicted here are a graphic representation and vary based on monitor or printer calibration. See color specifications.

ELECTRIC SIGN

This sign is intended to be installed in accordance with the requirements of Article 600 of the National Electrical Code and/or other applicable local codes. This includes proper grounding and bonding of the sign.

• CONSTRUCTION APPROVALS •

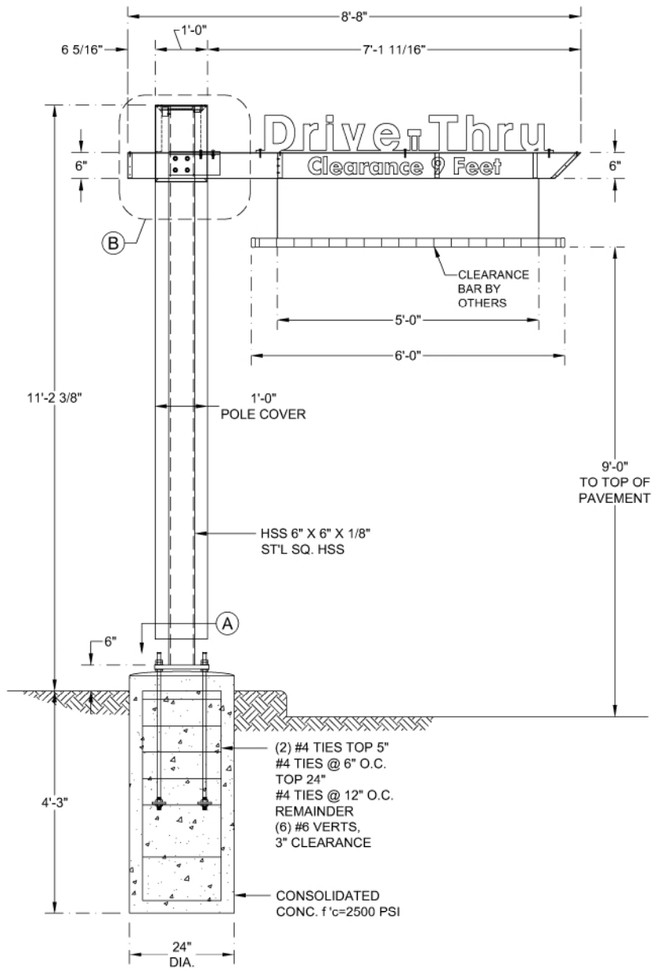
Acct. Mgr: Date:

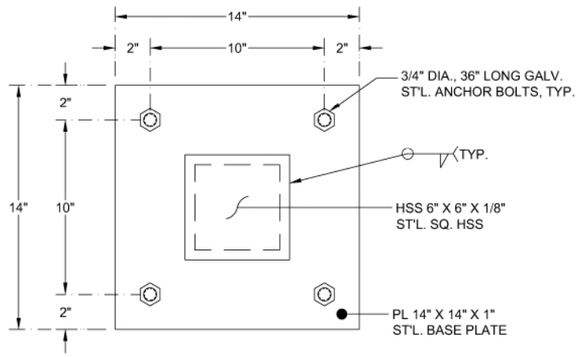
Design: Date:

Mfg/QC: Date:

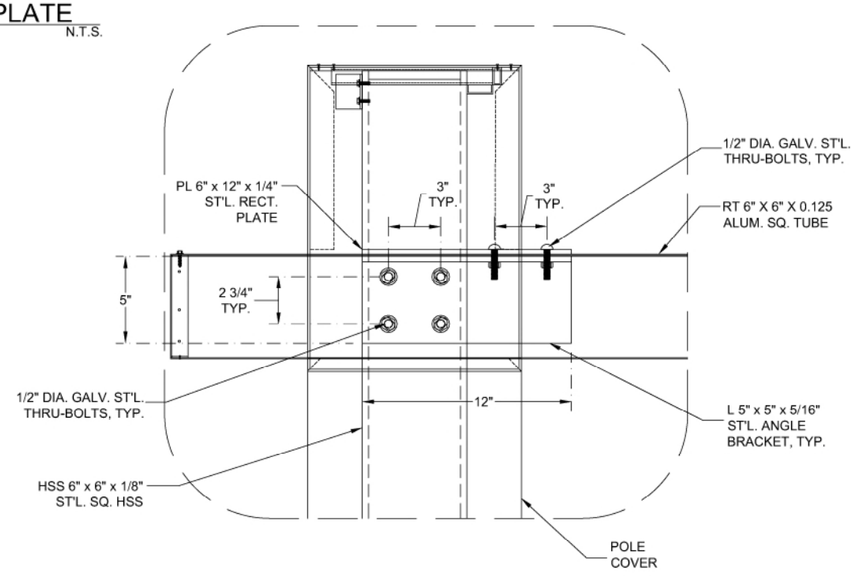
S-03

DRC Item 2





A BASE PLATE
t=1" N.T.S.



B CLEARANCE BAR CONNECTION DETAIL
NTS

Sign Design Based On 2022 CBC				
Job #	1029_117325			
Project	McDonald's - Clearance Bar			
Job Location	186 Niblick Rd.			
	Paso Robles, CA			
INPUT DATA				
Exposure category (B, C or D)		=	C	
Risk Category		=	II	
Ultimate Design Windspeed	V_{ULT}	=	120	MPH
Topographic factor	K_{zt}	=	1	Flat
Height of the sign	h	=	11.20	FT
Average Vertical dimension (for wall, s = h)	s	=	2.23	FT
Horizontal dimension	B	=	8.71	FT
Dimension of return corner	L_r	=	0.76	FT

ANALYSIS

Velocity pressure

$q_z = 0.00256\ K_z K_{zt} K_d V^2 K_e$ = **26.63** PSF

where:

q_z = velocity pressure at height h. (Eq. 26.10-1 page. 268)

K_z = velocity pressure exposure coefficient = **0.85**

evaluated at height above gRnd. level, h (Tab. 26.10-1, page 268)

K_d = wind directionality factor. (Tab. 26.6-1, page 266) = **0.85**

K_e = ground elevation factor, see (Tab. 26.9-1, page 268) = **1.00**

Wind Force Case A: resultant force through geometric center

Max horizontal wind pressure = $p = q_z G C_f$ = **41** PSF

where: G = gust effect factor. (Sec. 26.11-1, page 269). = **0.85**

C_f = net force coefficient. (Fig. 29.3-1, page 323) = **1.80**

A_s = B s = the gross area = **19.42** FT^2

Estimated sign cabinet weight = **118** LBS.

DESIGN SUMMARY

Allowable Stress Design Wind Factor =		**0.60**	
Design Wind Pressure =	**0.6 x p =**	**24.46**	PSF
Design Windforce, F =	**24.46 x As =**	**0.48**	KIPS
Moment Arm =		**8.36**	FT
Design Moment =	**F x Moment Arm =**	**3.97**	KIP-FT

Footing Design (Nonconstrained)

Diameter =	2.00	FT
Soil Pressure =	150.00	PSF/FT
S_1 =	420.00	PSF
A =	1.32	FT
EMBED. =	4.20	FT

24" DIA. DEPTH = 4' - 3"

Pole Design — ST'L. SQ. HSS

Sec. Mod. Req'd. S = 1.82 — USE HSS 6" x 6" x 1/8" — A500 GR. B Fy= 46000 PSI

Torsion Shear — Torsion = 848 LB-FT — S = 5.15 IN^3

τ = 901 — t = 0.17 IN^4

Shear Stress — b = 6.00 IN

V = 396.0 — A = 2.70 IN^2

Total V Stress= 1297 — allow fv = 18400

Unity = (1.82 / 5.15) + (1297 / 18400) = 0.42 **< 1 (OK)**

Base Plate — ST'L. PLATE

Thickness Req'd. t = 0.40 — USE PL 14" x 14" x 1" — A36 t = 1.00 **(OK)**

Anchor Design — GALV. STL. ANCHOR BOLT

Tension Req'd. T = 2383 — USE 3/4" DIA., x 36" LONG — F 1554 GR. 36 T = 9610

Shear Req'd. V = 148 — V = 5130

Unity = (2383 / 9610) + (148 / 5130) = 0.28 **< 1 (OK)**

Arm Design — ALUM. SQ. TUBE

Sec. Mod. Req'd. S = 0.48 — USE RT 6.0" x 6.0" x 0.125" — 6061-T6 S = 5.64 **(OK)**

Bolt Design — GALV. STL. THRU-BOLT

Tension Req'd. T = 2036 — USE 1/2" DIA., — A307 T = 4410

Shear Req'd. V = 60 — V = 2350

Unity = (2036 / 4410) + (60 / 2350) = 0.49 **< 1 (OK)**

REGISTERED PROFESSIONAL ENGINEER
TONY JACOB
C59022
CIVIL
STATE OF CALIFORNIA
May 22, 2025

NOTICE: IT IS A VIOLATION OF THE LAW FOR ANY PERSON, UNLESS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE SEAL OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING LICENSED PROFESSIONAL SHALL AFFIX TO THEIR ITEM THEIR SEAL AND THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE AND THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

NOTES :

GENERAL :
- SIGN DESIGN IS BASED ON ADEQUATE EXISTING SUPPORT ELEMENTS.
- PROVIDE ISOLATION OF DISSIMILAR MATERIALS.
- COAT ALUMINUM IN CONTACT WITH CONCRETE WITH ZINC RICH PAINT.
- PROVIDE FULLY WELDED END CAPS AT EXPOSED OPEN ENDS OF STEEL / ALUM. TUBES, MATCH THICKNESS LIKE FOR LIKE.
- SLOPE TOP OF EXPOSED FOOTING AWAY FROM DIRECT BURIAL POSTS
- ALL EXPOSED STEEL TO BE PRIMED & PAINTED (POWDER COAT AS AN OPTION) OR ALTERNATIVELY USE GALVANIZED STEEL.

STEEL :
DESIGN AND FABRICATION ACCORDING TO 2022 CBC
- PLATE, ANGLE, CHANNEL TEE: ASTM A36
- WIDE FLANGE: ASTM A992
- ROUND PIPE: ASTM A53 GRADE B OR EQUIVALENT.
- HSS ROUND, SQUARE, AND RECTANGULAR TUBE: ASTM A500 GRADE B OR EQUIVALENT.
- STAINLESS STEEL ROUND, SQUARE, AND RECTANGULAR TUBE: ASTM A276 T304 OR EQUIVALENT.
- ALL ANCHORS BOLTS SHALL BE: ASTM F1554 OR ASTM F593 T304 U.N.O.
- ALL STEEL MACHINED BOLTS SHALL BE: ASTM A307, A325 OR A449 U.N.O.
- ALL STAINLESS STEEL MACHINED BOLTS SHALL BE: ASTM F593 T304 U.N.O.
- ALL BOLTS TO BE ZINC COATED: ASTM B633
- DEFORMED REINFORCING REBAR: ASTM A615 GRADE 60.

STEEL
DESIGN AND FABRICATION ACCORDING TO AWS D1.1. / D1.3 & D1.6
- AWS CERTIFICATION REQUIRED FOR ALL STRUCTURAL WELDERS.

ALUMINUM :
DESIGN AND FABRICATION ACCORDING TO 2020 ALUM. DESIGN MANUAL

PLATES, ANGLES, CHANNELS, TEE, AND SQUARE TUBING: ALUMINUM
- ALLOY 6061 - T6 WITH 0.098 LBS PER CUBIC INCH.

ALUMINUM
DESIGN AND FABRICATION ACCORDING TO AWS D1.2. ALL WELDING IN ACCORDANCE WITH THE LATEST EDITION OF THE AWS A.5.10. FILLER ALLOYS PER TABLES M.9.1 & M.9.2 OF 2020 ALUMINUM DESIGN MANUAL.

WELDING :
- WELD SIZES SHALL BE EQUAL TO THE THICKNESS OF THE THINNEST MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
- E70 XX ELECTRODE FOR SMAW PROCESS.
- E70S XX ELECTRODE FOR GMAW PROCESS.
- ER7 XX ELECTRODE FOR GTAW PROCESS.
- E70T XX ELECTRODE FOR FCAW PROCESS.

ALL WELDS SHALL BE MADE WITH A FILLER METAL THAT CAN PRODUCE WELDS THAT HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20FT-LB AT ZERO 0° AS DETERMINED BY THE APPROPRIATE AWS A5 CLASSIFICATION TEST METHOD OR MFG'S. CERTIFICATION.

ANCHORS :
- BRAND NAME APPROVED ANCHORS SPECIFIED ON PLANS MAY BE SUBSTITUTED BY APPROVED EQUAL.

CONCRETE :
DESIGN AND CONSTRUCTION ACCORDING TO ACI 318-19
- COMPRESSIVE STRENGTH AT 28 DAYS, f 'c= 2500 PSI MINIMUM.
- CEMENT TYPE II OR IV. W/C RATIO 0.45 BY WEIGHT FOR PIER AND CAISSON
- FOOTINGS CONCRETE MUST BE POURED AGAINST UNDISTURBED EARTH.
- MAINTAIN A MINIMUM 3" CONCRETE COVER OVER ALL EMBEDDED STEEL.

SOIL:
LATERAL SOIL BEARING PER IBC CLASS 4 TABLE 1806.2 (150 PSF/FT). MODIFIED PER SECTION 1806.3.4.

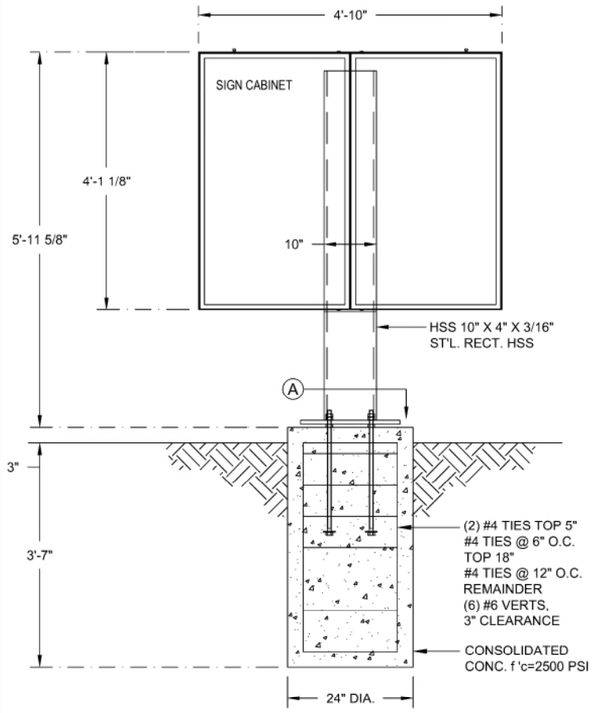
www.yjinc.com
P.O. BOX 802050
SANTA CLARITA, CA. 91380
TEL. (661)259-0700 FAX. (661)259-0900

SHEET TITLE:

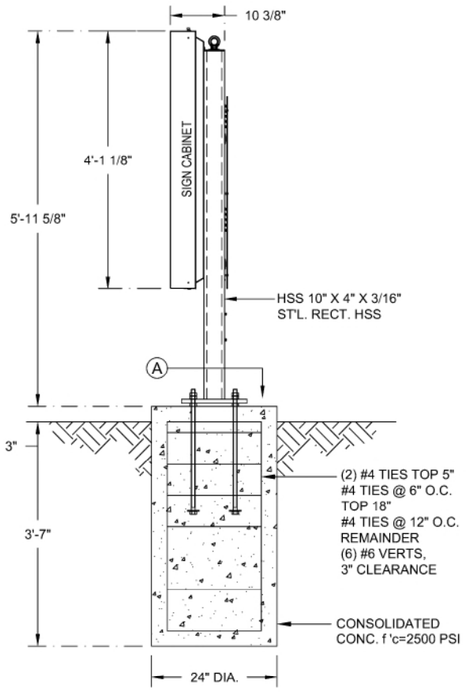
MCDONALD'S CLEARANCE BAR

DRN BY: J.J.	DATE LAST REVISED: May 22, 2025	PROJECT JOB # : 1029_ 117325_McDonalds Signage Niblick Rd Paso Robles CA.dwg			
CHK BY: T.J.	PROJ. START DATE: May 22, 2025	REV. NO.	REV. DATE	REVISED BY	PROJECT LOCATION : MCDONALD'S 186 NIBLICK RD. PASO ROBLES, CA
REV BY: T.J.	SCALE: AS SHOWN	1	--/--/--	---	SHEET # 1 OF 1
PLOTTED BY: Michelle	ON 5/22/2025 10:58:41 AM	2	--/--/--	---	

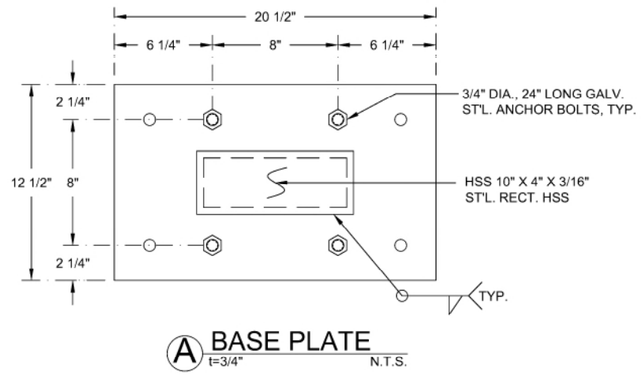
DRC Item 2



ELEVATION
N.T.S.



SIDE VIEW
N.T.S.



A BASE PLATE
t=3/4"
N.T.S.

Sign Design Based On 2022 CBC

Job #	1029_117325			
Project	McDonald's - DT Menu Board			
Job Location	186 Niblick Rd. Paso Robles, CA			
INPUT DATA				
Exposure category (B, C or D)		=	C	
Risk Category		=	II	
Ultimate Design Windspeed	V_{ULT}	=	120	MPH
Topographic factor	K_{zt}	=	1	Flat
Height of the sign	h	=	6.23	FT
Average Vertical dimension (for wall, s = h)	s	=	4.55	FT
Horizontal dimension	B	=	4.83	FT
Dimension of return corner	L_r	=	0.88	FT
ANALYSIS				
Velocity pressure				
$q_z = 0.00256\ K_z K_{zt} K_d V^2 K_e$		=	26.63	PSF
where:				
q_z = velocity pressure at height h. (Eq. 26.10-1 page. 268)				
K_z = velocity pressure exposure coefficient		=	0.85	
evaluated at height above gRnd. level, h (Tab. 26.10-1, page 268)				
K_d = wind directionality factor. (Tab. 26.6-1, page 266)		=	0.85	
K_e = ground elevation factor, see (Tab. 26.9-1, page 268)		=	1.00	
Wind Force Case A: resultant force through geometric center				
Max horizontal wind pressure =	$p = q_z GC_f =$	=	37	PSF
where: G = gust effect factor. (Sec. 26.11-1, page 269).		=	0.85	
C_f = net force coefficient. (Fig. 29.3-1, page 323)			1.63	
A_s = B s = the gross area		=	22.00	FT^2
Estimated sign cabinet weight		=	133	LBS.
DESIGN SUMMARY				
Allowable Stress Design Wind Factor =			0.60	
Design Wind Pressure =		**0.6 x p =**	22.16	PSF
Design Windforce, F =		**22.16 x As =**	0.49	KIPS
Moment Arm =			3.84	FT
Design Moment =		**F x Moment Arm =**	1.87	KIP-FT

Footing Design (Nonconstrained)

Diameter =	2.00	FT
Soil Pressure =	150.00	PSF/FT
S_1 =	353.00	PSF
A =	1.62	FT
EMBED. =	3.53	FT

24"	DIA.	DEPTH = 3' - 7"

Pole Design			STL. RECT. HSS			
Sec. Mod. Req'd.			USE	A500 GR. B		
Sy = 0.81	HSS	10" x	4" x	3/16"	Sy = 6.93	(OK)
Base Plate			STL. PLATE			
Thickness Req'd.			USE	A36		
t = 0.25	PL	20 1/2" x	12 1/2" x	3/4"	t = 0.75	(OK)
Anchor Design			GALV. STL. ANCHOR BOLT			
Tension Req'd.			USE	F 1554 GR. 36		
T = 1404	3/4" DIA., x	24" LONG			T = 9610	
Shear Req'd.						
V = 155					V = 5130	
Unity =	(1404 / 9610) +	(155 /	5130) = 0.18		< 1 (OK)	

REGISTERED PROFESSIONAL ENGINEER
TONY JACOB
C59022
CIVIL
STATE OF CALIFORNIA
May 22, 2025

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- COAT ALUMINUM IN CONTACT WITH CONCRETE WITH ZINC RICH PAINT.
- PROVIDE FULLY WELDED END CAPS AT EXPOSED OPEN ENDS OF STEEL / ALUM. TUBES, MATCH THICKNESS LIKE FOR LIKE.
- SLOPE TOP OF EXPOSED FOOTING AWAY FROM DIRECT BURIAL POSTS
- ALL EXPOSED STEEL TO BE PRIMED & PAINTED (POWDER COAT AS AN OPTION) OR ALTERNATIVELY USE GALVANIZED STEEL.

STEEL :
DESIGN AND FABRICATION ACCORDING TO 2022 CBC
- PLATE, ANGLE, CHANNEL TEE: ASTM A36
- WIDE FLANGE: ASTM A992
- ROUND PIPE: ASTM A53 GRADE B OR EQUIVALENT.
- HSS ROUND, SQUARE, AND RECTANGULAR TUBE: ASTM A500 GRADE B OR EQUIVALENT.
- STAINLESS STEEL ROUND, SQUARE, AND RECTANGULAR TUBE: ASTM A276 T304 OR EQUIVALENT.
- ALL ANCHORS BOLTS SHALL BE: ASTM F1554 OR ASTM F593 T304 U.N.O.
- ALL STEEL MACHINED BOLTS SHALL BE: ASTM A307, A325 OR A449 U.N.O.
- ALL STAINLESS STEEL MACHINED BOLTS SHALL BE: ASTM F593 T304 U.N.O.
- ALL BOLTS TO BE ZINC COATED: ASTM B633
- DEFORMED REINFORCING REBAR: ASTM A615 GRADE 60.

STEEL
DESIGN AND FABRICATION ACCORDING TO AWS D1.1. / D1.3 & D1.6
- AWS CERTIFICATION REQUIRED FOR ALL STRUCTURAL WELDERS.

ALUMINUM :
DESIGN AND FABRICATION ACCORDING TO 2020 ALUM. DESIGN MANUAL

PLATES, ANGLES, CHANNELS, TEE, AND SQUARE TUBING: ALUMINUM
- ALLOY 6061 - T6 WITH 0.098 LBS PER CUBIC INCH.

ALUMINUM
DESIGN AND FABRICATION ACCORDING TO AWS D1.2. ALL WELDING IN ACCORDANCE WITH THE LATEST EDITION OF THE AWS A.5.10. FILLER ALLOYS PER TABLES M.9.1 & M.9.2 OF 2020 ALUMINUM DESIGN MANUAL.

WELDING :
- WELD SIZES SHALL BE EQUAL TO THE THICKNESS OF THE THINNEST MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
- E70 XX ELECTRODE FOR SMAW PROCESS.
- E70S XX ELECTRODE FOR GMAW PROCESS.
- ER7 XX ELECTRODE FOR GTAW PROCESS.
- E70T XX ELECTRODE FOR FCAW PROCESS.

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ANCHORS :
- BRAND NAME APPROVED ANCHORS SPECIFIED ON PLANS MAY BE SUBSTITUTED BY APPROVED EQUAL.

CONCRETE :
DESIGN AND CONSTRUCTION ACCORDING TO ACI 318-19
- COMPRESSIVE STRENGTH AT 28 DAYS, f 'c= 2500 PSI MINIMUM.
- CEMENT TYPE II OR IV. W/C RATIO 0.45 BY WEIGHT FOR PIER AND CAISSON
- FOOTINGS CONCRETE MUST BE POURED AGAINST UNDISTURBED EARTH.
- MAINTAIN A MINIMUM 3" CONCRETE COVER OVER ALL EMBEDDED STEEL.

SOIL:
LATERAL SOIL BEARING PER IBC CLASS 4 TABLE 1806.2 (150 PSF/FT). MODIFIED PER SECTION 1806.3.4.

www.yjinc.com
P.O. BOX 802050
SANTA CLARITA, CA. 91380
TEL. (661)259-0700 FAX. (661)259-0900

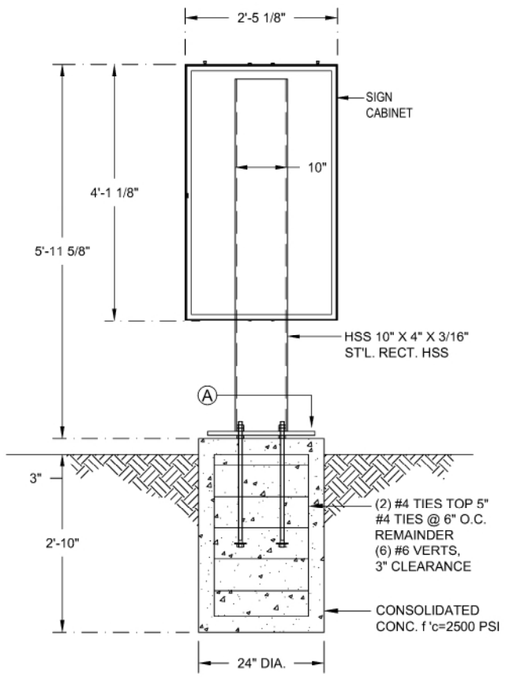
SHEET TITLE:

MCDONALD'S
DT MENUBOARD

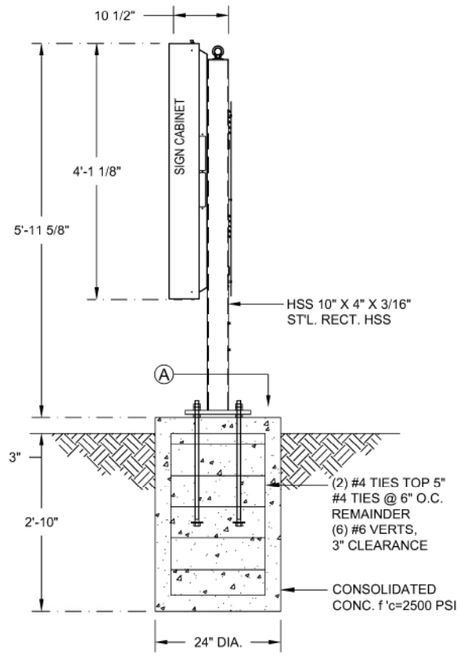
DRN BY: J.J.	DATE LAST REVISED: May 22, 2025	PROJECT JOB # : 1029_ 117325_McDonalds Signage Niblick Rd Paso Robles CA.dwg			
CHK BY: T.J.	PROJ. START DATE: May 22, 2025	REV. NO.	REV. DATE	REVISED BY	PROJECT LOCATION : MCDONALD'S 186 NIBLICK RD. PASO ROBLES, CA
REV BY: T.J.	SCALE: AS SHOWN	1	--/--/--	---	
PLOTTED BY: Michelle	ON 5/22/2025 10:58:42 AM	2	--/--/--	---	

SHEET # 1 OF 1

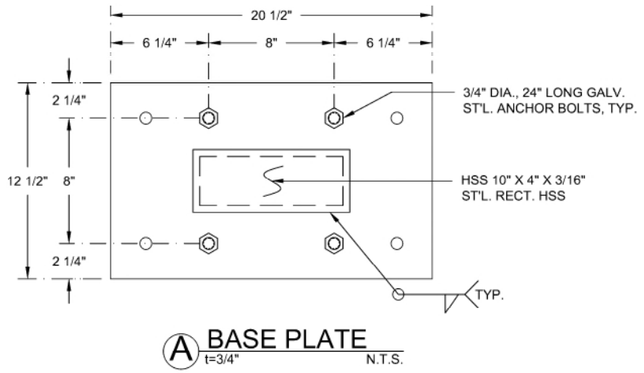
DRC Item 2



ELEVATION
N.T.S.



SIDE VIEW
N.T.S.



A BASE PLATE
t=3/4"
N.T.S.

Sign Design Based On 2022 CBC

Job #	1029_117325			
Project	McDonald's - DT Pre-Sell			
Job Location	186 Niblick Rd. Paso Robles, CA			
INPUT DATA				
Exposure category (B, C or D)		=	C	
Risk Category		=	II	
Ultimate Design Windspeed	V_{ULT}	=	120	MPH
Topographic factor	K_{zt}	=	1	Flat
Height of the sign	h	=	6.24	FT
Average Vertical dimension (for wall, s = h)	s	=	5.01	FT
Horizontal dimension	B	=	2.43	FT
Dimension of return corner	L_r	=	0.88	FT
ANALYSIS				
Velocity pressure				
$q_z = 0.00256\ K_z K_{zt} K_d V^2 K_e$		=	26.63	PSF
where:				
q_z = velocity pressure at height h. (Eq. 26.10-1 page. 268)				
K_z = velocity pressure exposure coefficient		=	0.85	
evaluated at height above gRnd. level, h (Tab. 26.10-1, page 268)				
K_d = wind directionality factor. (Tab. 26.6-1, page 266)		=	0.85	
K_e = ground elevation factor, see (Tab. 26.9-1, page 268)		=	1.00	
Wind Force Case A: resultant force through geometric center				
Max horizontal wind pressure =	**$p = q_z G C_f$ =**	=	37	PSF
where: G = gust effect factor. (Sec. 26.11-1, page 269).		=	0.85	
C_f = net force coefficient. (Fig. 29.3-1, page 323)			1.65	
A_s = B s = the gross area		=	12.17	FT^2
Estimated sign cabinet weight		=	74	LBS.
DESIGN SUMMARY				
Allowable Stress Design Wind Factor =			0.60	
Design Wind Pressure =	**0.6 x p =**		22.44	PSF
Design Windforce, F =	**22.44 x As =**		0.27	KIPS
Moment Arm =			3.59	FT
Design Moment =	**F x Moment Arm =**		0.98	KIP-FT

Footing Design (Nonconstrained)

Diameter =	2.00	FT
Soil Pressure =	150.00	PSF/FT
S_1 =	277.00	PSF
A =	1.15	FT
EMBED. =	2.78	FT

24" DIA. DEPTH = 2' - 10"

Pole Design			STL. RECT. HSS			
Sec. Mod. Req'd.			USE	A500 GR. B		
Sy = 0.43	HSS 10" x	4" x	3/16"	Sy = 6.93		(OK)
Base Plate			STL. PLATE			
Thickness Req'd.			USE	A36		
t = 0.18	PL 20 1/2" x	12 1/2" x	3/4"	t = 0.75		(OK)
Anchor Design			GALV. STL. ANCHOR BOLT			
Tension Req'd.			USE	F 1554 GR. 36		
T = 735	3/4" DIA., x	24" LONG		T = 9610		
Shear Req'd.						
V = 87				V = 5130		
Unity =	(735 / 9610) +	(87 /	5130) = 0.09	< 1 (OK)		

TONY JACOB
REGISTERED PROFESSIONAL ENGINEER
C59022
CIVIL
STATE OF CALIFORNIA
May 22, 2025

NOTES :

GENERAL :
- SIGN DESIGN IS BASED ON ADEQUATE EXISTING SUPPORT ELEMENTS.
- PROVIDE ISOLATION OF DISSIMILAR MATERIALS.
- COAT ALUMINUM IN CONTACT WITH CONCRETE WITH ZINC RICH PAINT.
- PROVIDE FULLY WELDED END CAPS AT EXPOSED OPEN ENDS OF STEEL / ALUM. TUBES, MATCH THICKNESS LIKE FOR LIKE.
- SLOPE TOP OF EXPOSED FOOTING AWAY FROM DIRECT BURIAL POSTS
- ALL EXPOSED STEEL TO BE PRIMED & PAINTED (POWDER COAT AS AN OPTION) OR ALTERNATIVELY USE GALVANIZED STEEL.

STEEL :

DESIGN AND FABRICATION ACCORDING TO 2022 CBC
- PLATE, ANGLE, CHANNEL TEE: ASTM A36
- WIDE FLANGE: ASTM A992
- ROUND PIPE: ASTM A53 GRADE B OR EQUIVALENT.
- HSS ROUND, SQUARE, AND RECTANGULAR TUBE: ASTM A500 GRADE B OR EQUIVALENT.
- STAINLESS STEEL ROUND, SQUARE, AND RECTANGULAR TUBE: ASTM A276 T304 OR EQUIVALENT.
- ALL ANCHORS BOLTS SHALL BE: ASTM F1554 OR ASTM F593 T304 U.N.O.
- ALL STEEL MACHINED BOLTS SHALL BE: ASTM A307, A325 OR A449 U.N.O.
- ALL STAINLESS STEEL MACHINED BOLTS SHALL BE: ASTM F593 T304 U.N.O.
- ALL BOLTS TO BE ZINC COATED: ASTM B633
- DEFORMED REINFORCING REBAR: ASTM A615 GRADE 60.

STEEL
DESIGN AND FABRICATION ACCORDING TO AWS D1.1. / D1.3 & D1.6
- AWS CERTIFICATION REQUIRED FOR ALL STRUCTURAL WELDERS.

ALUMINUM :

DESIGN AND FABRICATION ACCORDING TO 2020 ALUM. DESIGN MANUAL

PLATES, ANGLES, CHANNELS, TEE, AND SQUARE TUBING: ALUMINUM
- ALLOY 6061 - T6 WITH 0.098 LBS PER CUBIC INCH.

ALUMINUM

DESIGN AND FABRICATION ACCORDING TO AWS D1.2. ALL WELDING IN ACCORDANCE WITH THE LATEST EDITION OF THE AWS A.5.10. FILLER ALLOYS PER TABLES M.9.1 & M.9.2 OF 2020 ALUMINUM DESIGN MANUAL.

WELDING :
- WELD SIZES SHALL BE EQUAL TO THE THICKNESS OF THE THINNEST MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
- E70 XX ELECTRODE FOR SMAW PROCESS.
- E70S XX ELECTRODE FOR GMAW PROCESS.
- ER7 XX ELECTRODE FOR GTAW PROCESS.
- E70T XX ELECTRODE FOR FCAW PROCESS.

ALL WELDS SHALL BE MADE WITH A FILLER METAL THAT CAN PRODUCE WELDS THAT HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20FT-LB AT ZERO 0° AS DETERMINED BY THE APPROPRIATE AWS A5 CLASSIFICATION TEST METHOD OR MFG'S. CERTIFICATION.

ANCHORS :
- BRAND NAME APPROVED ANCHORS SPECIFIED ON PLANS MAY BE SUBSTITUTED BY APPROVED EQUAL.

CONCRETE :

DESIGN AND CONSTRUCTION ACCORDING TO ACI 318-19
- COMPRESSIVE STRENGTH AT 28 DAYS, f 'c= 2500 PSI MINIMUM.
- CEMENT TYPE II OR IV. W/C RATIO 0.45 BY WEIGHT FOR PIER AND CAISSON
- FOOTINGS CONCRETE MUST BE POURED AGAINST UNDISTURBED EARTH.
- MAINTAIN A MINIMUM 3" CONCRETE COVER OVER ALL EMBEDDED STEEL.

SOIL:
LATERAL SOIL BEARING PER IBC CLASS 4 TABLE 1806.2 (150 PSF/FT). MODIFIED PER SECTION 1806.3.4.

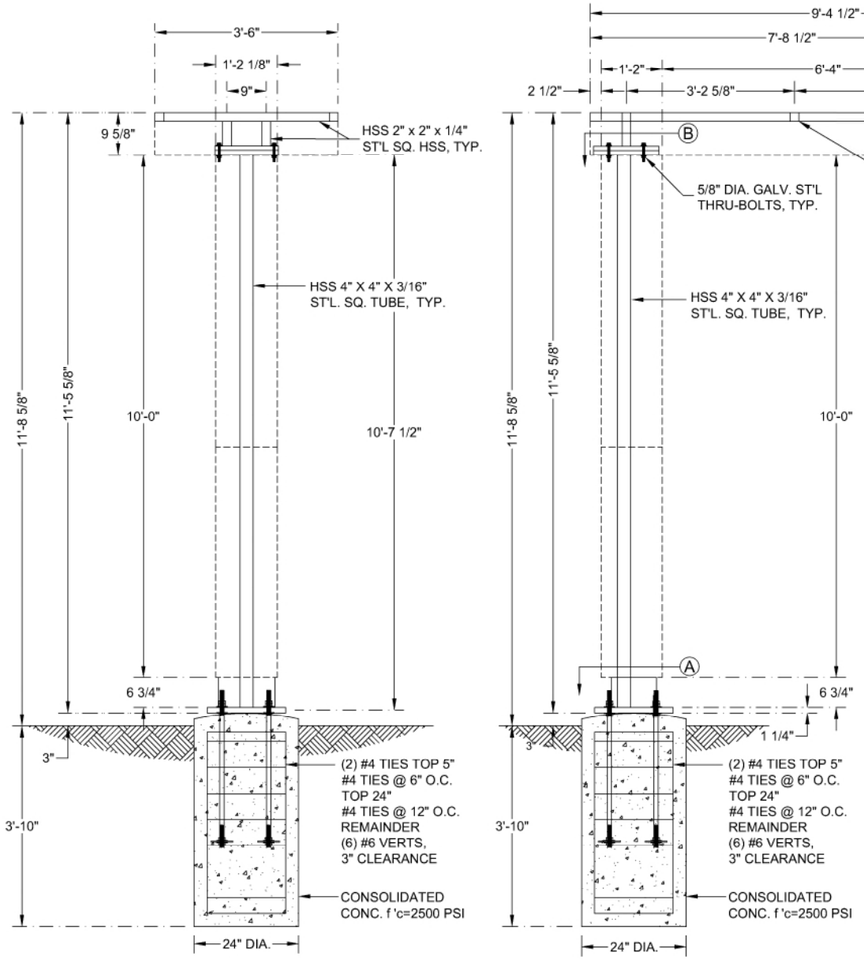
www.yjinc.com
P.O. BOX 802050
SANTA CLARITA, CA. 91380
TEL. (661)259-0700 FAX. (661)259-0900

SHEET TITLE:

MCDONALD'S
DT PRE-SELL

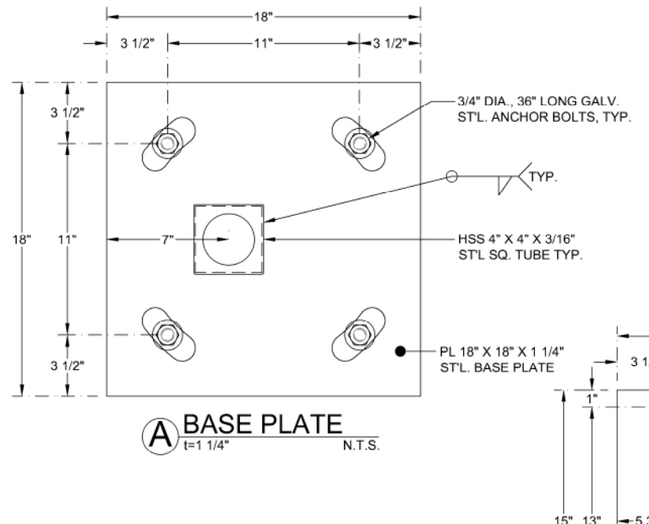
DRN BY: J.J.	DATE LAST REVISED: May 22, 2025	PROJECT JOB # : 1029_	117325_McDonalds Signage Niblick Rd Paso Robles CA.dwg		
CHK BY: T.J.	PROJ. START DATE: May 22, 2025	REV. NO.	REV. DATE	REVISED BY	PROJECT LOCATION : MCDONALD'S 186 NIBLICK RD. PASO ROBLES, CA
REV BY: T.J.	SCALE: AS SHOWN	1	--/--/--	---	SHEET # 1 OF 1
PLOTTED BY: Michelle	ON 5/22/2025 10:58:43 AM	2	--/--/--	---	

DRC Item 2

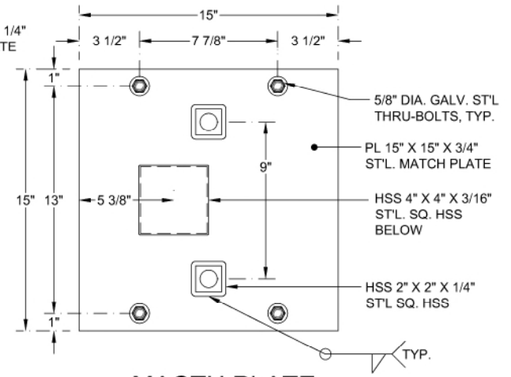


SIDE VIEW

ELEVATION



A BASE PLATE
t=1 1/4" N.T.S.



B MACTH PLATE
t=3/4" N.T.S.

Sign Design Based On 2022 CBC

Job #	1029_117325
Project	McDonald's - Slim Springboard Cod Canopy
Job Location	186 Niblick Rd.
	Paso Robles, CA

INPUT DATA

Exposure category (B, C or D)		=	C	
Risk Category		=	II	
Nominal Design Windspeed	V_{ULT}	=	120	MPH
Topographic factor	K_{zt}	=	1	Flat
Height of the sign	h	=	11.72	FT
Vertical dimension (for wall, s = h)	s	=	11.72	FT
Horizontal dimension	B	=	2.58	FT
Dimension of return corner	L_r	=	0.75	FT

ANALYSIS

Velocity pressure

$q_h = 0.00256\ K_h\ K_{zt}\ K_d\ V^2\ K_e$ = 26.63 psf

where:

qh = velocity pressure at height h. (Eq. 29.3-1, pg. 322)

K_h = velocity pressure exposure coefficient = 0.85

evaluated at height above ground level, h (Tab. 26.10-1, pg. 268)

K_d = wind directionality factor. (Tab. 26.6-1, pg. 266) = 0.85

K_e = For all elev. See (Tab. 29.4-1, page 268) = 1.00

Wind Force Case A: resultant force though geometric center (Sec. 29.4.1 & Fig. 29.4-1)

Max horizontal wind pressure = $p = q_h\ GC_f$ = 37.20 PSF

where: G = gust effect factor. (Sec. 26.11.1, pg. 269) = 0.85

Cf = net force coefficient. (Fig. 29.3-1, pg. 323) 1.64

A_s = B s = the gross area = 30.2 FT^2

DESIGN SUMMARY

Allowable Stress Design Wind Factor =	0.6	
Design Wind Pressure = 0.6 x p =	22.32	PSF
Design Windforce, F = 22.32 x As =	0.675	KIPS
Moment Arm =	6.02	FT
Design Moment = F x moment arm =	4.061	KIP-FT

Canopy Frame Design *(See attached Risa calcs)*

ST'L. SQ. HSS

USE A500 Grade B

HSS 2" X 2" X 1/4"

Pole Design *(See attached Risa calcs)*

ST'L. SQ. HSS

USE A500 Grade B

HSS 4" X 4" X 3/16"

Footing Design (Nonconstrained)

Max. Support Moment reaction, LC5, Node N16A, MZ =- 3.507 KIP-FT

Shear, X = 0.237 KIPS

Moment Arm = 14.80 FT

Diameter =	2.00	FT
Soil Pressure =	150.00	PSF/FT
S_1 =	381.00	PSF
A =	0.73	FT
EMBED. =	3.81	FT

24 in. Dia. Depth = 3' - 10"

Anchor Design GALV. STL . ANCHOR BOLTS

Tension Req'd. USE ASTM F1554 Grade 36

T = 1913 **3/4" DIA., 36" LONG** T = 9610

Base Plate ST'L. PLATE

Thickness Req'd. USE A36

Thick = 0.44 **18" x 18" x 1 1/4"** Thick = 1.25

Bolt Design

Max. Member End Moment reaction, LC2, M8, Mz= 3.507 KIP-FT

Shear, y =- 0.970 KIPS

Shear, z = 0.000 KIPS

Tension, x = 0.870 KIPS

Resultant Shear = 0.970 KIPS

GALV. STL . THRU- BOLTS

Tension Required USE A307

T = 6230 **5/8" DIA.** T = 6910

Shear Required

V = 243 V = 3680

Unity = (6230 / 6910) + (243 / 3680) = 0.97 < 1 (OK)

Match Plate STL. PLATE

Thickness Req'd. USE A36

t= 0.38 **15" X 15" X 3/4"** t= 0.75

NOTICE: IT IS A VIOLATION OF THE LAW FOR ANY PERSON, UNLESS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE SEAL OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING LICENSED PROFESSIONAL SHALL AFFIX TO THEIR ITEM THEIR SEAL AND THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE AND THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

NOTES :

GENERAL :

- SIGN DESIGN IS BASED ON ADEQUATE EXISTING SUPPORT ELEMENTS.
- PROVIDE ISOLATION OF DISSIMILAR MATERIALS.
- COAT ALUMINUM IN CONTACT WITH CONCRETE WITH ZINC RICH PAINT.
- PROVIDE FULLY WELDED END CAPS AT EXPOSED OPEN ENDS OF STEEL / ALUM. TUBES, MATCH THICKNESS LIKE FOR LIKE.
- SLOPE TOP OF EXPOSED FOOTING AWAY FROM DIRECT BURIAL POSTS
- ALL EXPOSED STEEL TO BE PRIMED & PAINTED (POWDER COAT AS AN OPTION) OR ALTERNATIVELY USE GALVANIZED STEEL.

STEEL :

DESIGN AND FABRICATION ACCORDING TO 2022 CBC

- PLATE, ANGLE, CHANNEL TEE: ASTM A36
- WIDE FLANGE: ASTM A992
- ROUND PIPE: ASTM A53 GRADE B OR EQUIVALENT.
- HSS ROUND, SQUARE, AND RECTANGULAR TUBE: ASTM A500 GRADE B OR EQUIVALENT.
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- ALL BOLTS TO BE ZINC COATED: ASTM B633
- DEFORMED REINFORCING REBAR: ASTM A615 GRADE 60.

STEEL

DESIGN AND FABRICATION ACCORDING TO AWS D1.1. / D1.3 & D1.6

- AWS CERTIFICATION REQUIRED FOR ALL STRUCTURAL WELDERS.

ALUMINUM :

DESIGN AND FABRICATION ACCORDING TO 2020 ALUM. DESIGN MANUAL

PLATES, ANGLES, CHANNELS, TEE, AND SQUARE TUBING: ALUMINUM

- ALLOY 6061 - T6 WITH 0.098 LBS PER CUBIC INCH.

ALUMINUM

DESIGN AND FABRICATION ACCORDING TO AWS D1.2. ALL WELDING IN ACCORDANCE WITH THE LATEST EDITION OF THE AWS A.5.10. FILLER ALLOYS PER TABLES M.9.1 & M.9.2 OF 2020 ALUMINUM DESIGN MANUAL.

WELDING :

- WELD SIZES SHALL BE EQUAL TO THE THICKNESS OF THE THINNEST MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.
- E70 XX ELECTRODE FOR SMAW PROCESS.
- E70S XX ELECTRODE FOR GMAW PROCESS.
- ER7 XX ELECTRODE FOR GTAW PROCESS.
- E70T XX ELECTRODE FOR FCAW PROCESS.

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ANCHORS :

- BRAND NAME APPROVED ANCHORS SPECIFIED ON PLANS MAY BE SUBSTITUTED BY APPROVED EQUAL.

CONCRETE :

DESIGN AND CONSTRUCTION ACCORDING TO ACI 318-19

- COMPRESSIVE STRENGTH AT 28 DAYS, f 'c= 2500 PSI MINIMUM.
- CEMENT TYPE II OR IV. W/C RATIO 0.45 BY WEIGHT FOR PIER AND CAISSON
- FOOTINGS CONCRETE MUST BE POURED AGAINST UNDISTURBED EARTH.
- MAINTAIN A MINIMUM 3" CONCRETE COVER OVER ALL EMBEDDED STEEL.

SOIL:

LATERAL SOIL BEARING PER IBC CLASS 4 TABLE 1806.2 (150 PSF/FT). MODIFIED PER SECTION 1806.3.4.

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P.O. BOX 802050
SANTA CLARITA, CA. 91380
TEL. (661)259-0700 FAX. (661)259-0900

SHEET TITLE:

MCDONALD'S
SLIM SPRINGBOARD COD CANOPY

DRN BY: J.J.	DATE LAST REVISED: May 22, 2025	PROJECT JOB # : 1029_ 117325_McDonalds Signage Niblick Rd Paso Robles CA.dwg			
CHK BY: T.J.	PROJ. START DATE: May 22, 2025	REV. NO.	REV. DATE	REVISED BY	PROJECT LOCATION : MCDONALD'S 186 NIBLICK RD. PASO ROBLES, CA
REV BY: T.J.	SCALE: AS SHOWN	1	--/--/--	---	
PLOTTED BY: Michelle ON 5/22/2025 10:58:43 AM		2	--/--/--	---	

SHEET # 1 OF 1