



**CITY OF EL PASO DE ROBLES**  
**"The Pass of the Oaks"**  
**Development Review Committee Agenda**

Monday, April 28, 2025, 3:30 PM  
Large Conference Room - 2nd Floor  
1000 SPRING ST  
Paso Robles, CA 93446

This is an in-person meeting. Written public comments can be submitted via email to [planning@prcity.com](mailto:planning@prcity.com). Those received prior to 12:00 noon on the day of the meeting to be posted to the City's website as an addendum to the agenda. If submitting written comments in advance of the meeting, please note the agenda item by number or name.

**AMERICANS WITH DISABILITIES ACT**

Any individual, who because of a disability needs special assistance to attend or participate in this meeting, may request assistance by contacting the City Clerk's Office (805) 237-3960. Whenever possible, requests should be made four (4) working days in advance of the meeting.

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	Pages
<b>A. CALL TO ORDER</b>	
<b>B. ROLL CALL</b>	
<b>C. DISCUSSION ITEMS</b>	
1. Item 1	3
File #: P25-0013	
Requested Action: DRC Final Action	
Application: Existing pole sign face change for Bubba's Smokehouse & Spirits	
Location: 1125 24 <sup>th</sup> Street	
Applicant: Roger Sharp	
2. Item 2	6
File #: P25-0007	
Requested Action: DRC Final Action	
Application: New wall-mounted sign for Vina Robles	
Location: 1650 Ramada Drive	
Applicant: Vina Robles (Nina Leschinsky)	
3. Item 3	10
File #: B25-0151	
Requested Action: DRC Final Action	
Application: New metal storage building	
Location: 1509 N River Rd	
Applicant: Paul Viborg	
4. Item 4	36
File #: P25-0039	

**Requested Action:** DRC Final Action

**Application:** Fence height modification request for the front yard

**Location:** 422 17<sup>th</sup> Street

**Applicant:** Vanessa Aispuro

**D. ADJOURNMENT**

EXISTITNG SIGN FRAME  
RUST FINISH  
67.7 Square Feet

BUBBA'S LOGO WHITE  
HALO LIGHTING  
PAINTED RED STAINLESS  
LETTERS

RED 3" DEEP CHANNEL  
ARROWS, ALWAYS "ON"  
NO CHASE.  
LOW VOLTAGE PLASTIC  
LED EDISON BULBS

ALUMINUM .060 FABRICATION  
POWDER COATED PRIMER  
160' Linear Frontage on  
24th Street

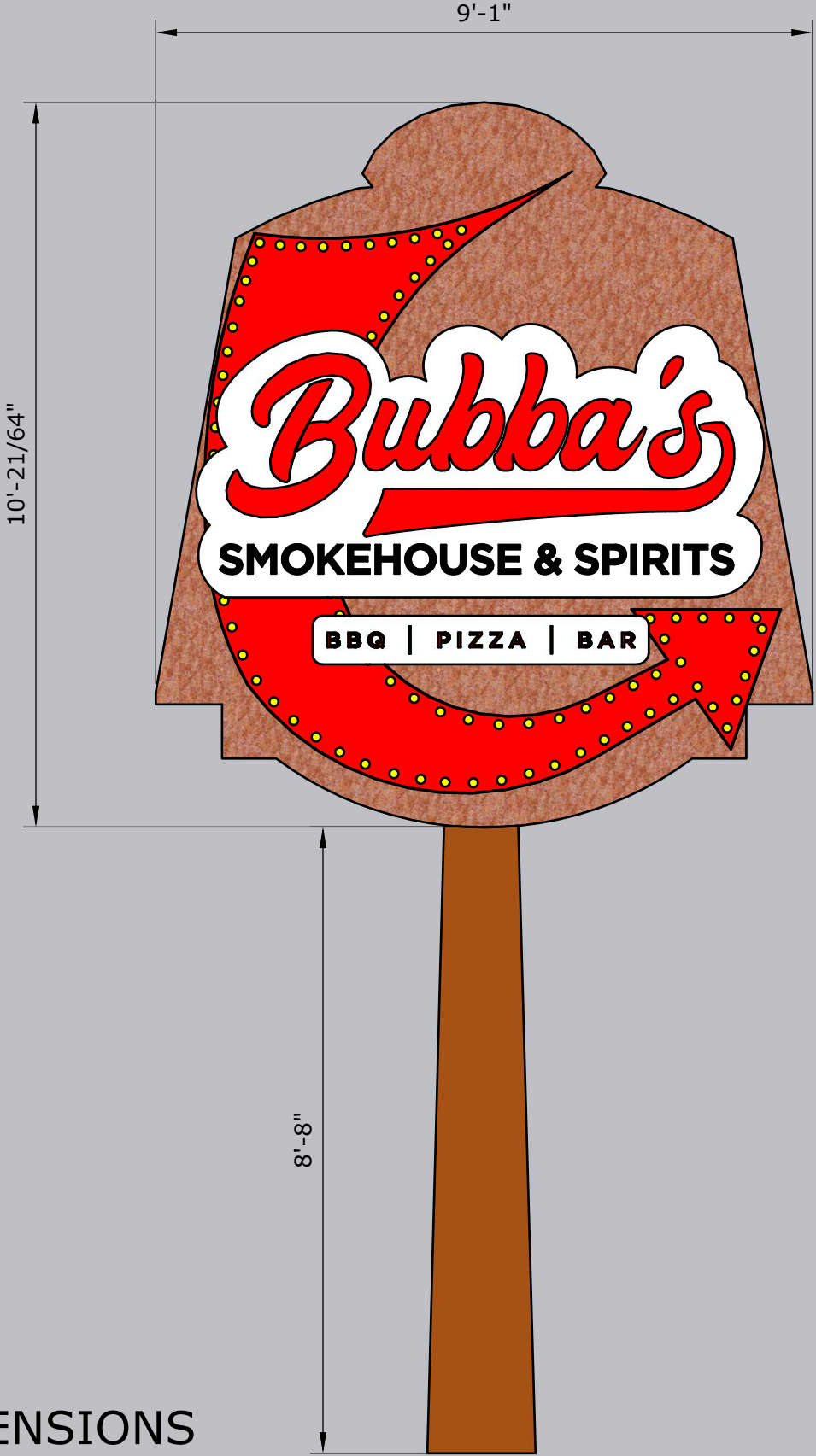


BLACK  
"SMOKEHOUSE & SPIRITS"

RED  
"BBQ | PIZZA | BAR"  
With pale yellow BG

EXISTING WOOD  
CLAD STEEL POST

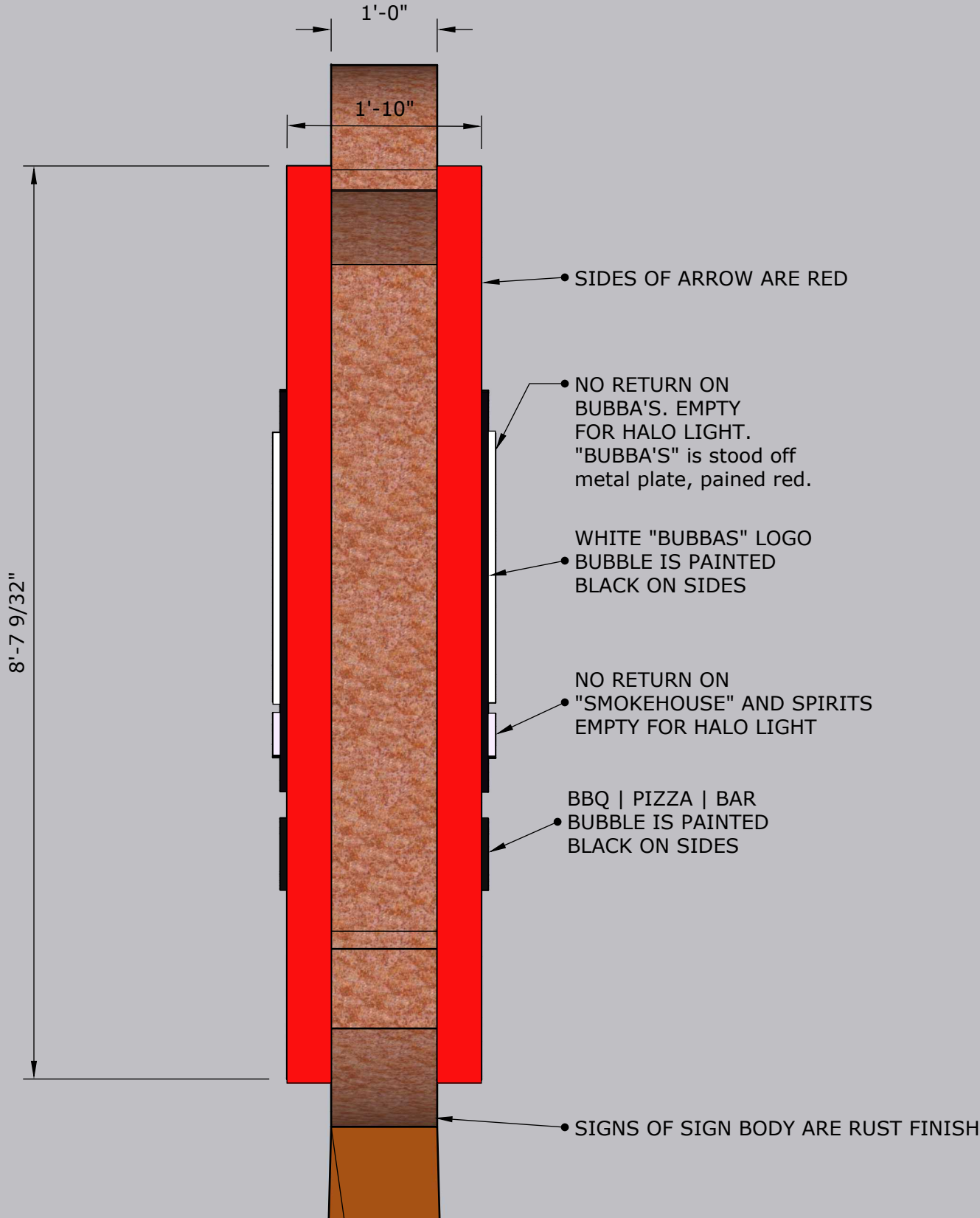
**BIG BUBBAS BAD BBQ SIGN FACE MODIFICATION, 1125 24th St., PASO ROBLES, CA**



SIGN DIMENSIONS



SIDE VIEW



# VINA ROBLES VINEYARDS & WINERY

1650 Ramada Dr Suite 140, Paso Robles, CA 93446



A

NON-LIT 3/8" FLAT CUT  
WHITE ACRYLIC  
LETTERS

# VINA ROBLES VINEYARDS & WINERY

1650 Ramada Dr Suite 140, Paso Robles, CA 93446

## ELEVATION A

16.7" X 150" = 17.4 SQ/FT



VINA ROBLES

HEIGHT OF SUITE 246"

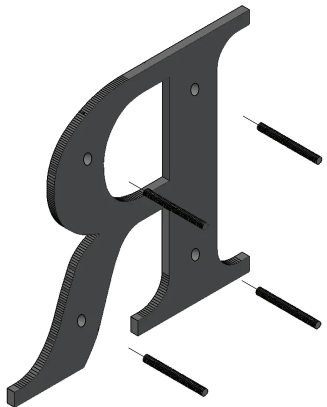
# VINA ROBLES VINEYARDS & WINERY

1650 Ramada Dr Suite 140, Paso Robles, CA 93446

## ELEVATION A

16.7" X 150" = 17.4 SQ/FT

### 3/8" NON-LIT WHITE ACRYLIC LETTERS



LETTERS WILL COME  
WITH THREADED  
STUDS FOR MOUNTING



## VINA ROBLES VINEYARDS & WINERY

1650 Ramada Dr Suite 140, Paso Robles, CA 93446

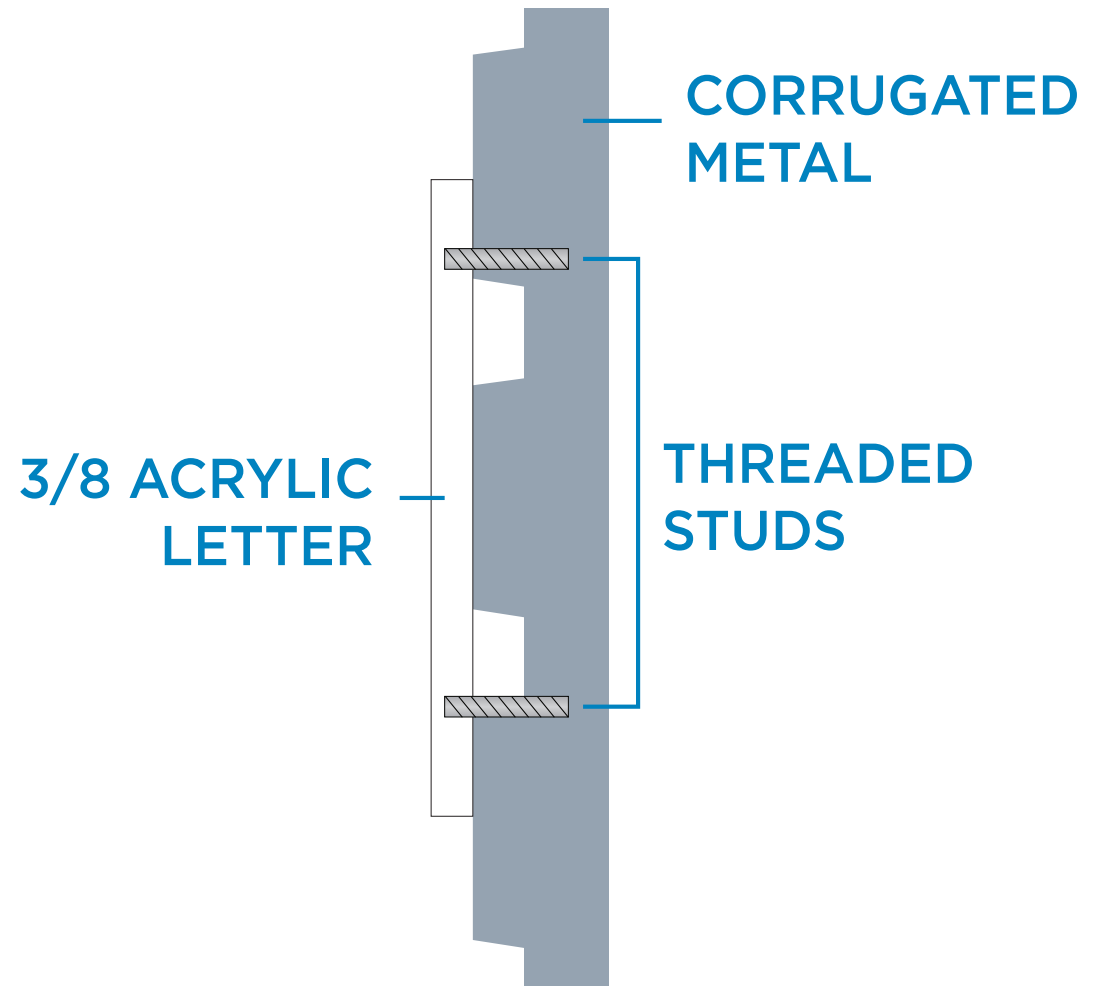
## ELEVATION A

16.7" X 150" = 17.4 SQ/FT

## INSTALLATION NOTES

**NON-LIT 3/8" ACRYLIC  
LETTERS WILL BE  
ATTACHED TO THE  
CORRUGATED METAL  
EXTERIOR WALL USING  
THREADED STUDS**

**CLEAR LEXEL WILL BE  
SUNK INTO HOLES TO  
CREATE WATERTIGHT  
SEAL**





EROSION CONTROL

1. EROSION AND SEDIMENT CONTROL BEST MANAGEMENT PRACTICES MUST BE IN PLACE AND FUNCTIONAL PRIOR TO THE FIRST INSPECTION. NO INSPECTIONS CAN BE PERFORMED IF THEY ARE NOT IN PLACE OR HAVE FAILED TO PROVIDE EROSION CONTROL. FAILURE TO MAINTAIN EROSION CONTROL WILL CAUSE INSPECTIONS TO BE DELAYED UNTIL EROSION CONTROL MEASURES ARE FUNCTIONAL.
2. EROSION CONTROL MEASURES SHALL BE IMPLEMENTED AND MAINTAINED TO THE SATISFACTION OF THE BUILDING OFFICIAL AND PUBLIC WORKS DIRECTOR DURING ALL DEMOLITIONS, CONSTRUCTION AND GROUND DISTURBING ACTIVITIES
3. THE ADJOINING STREET SHALL BE CLEANED BY SWEEPING TO REMOVE DIRT, DUST, MUD AND CONSTRUCTION DEBRIS AT THE END OF EACH DAY.
4. TEMPORARY EROSION CONTROL MEASURES SHALL BE REMOVED WHEN PERMANENT IMPROVEMENTS, PLANTINGS AND FACILITIES ARE IN PLACE. TEMPORARY MEASURES SHALL BE REMOVED PRIOR TO FINAL INSPECTION APPROVALS
5. THE FOLLOWING PERSON SHALL BE RESPONSIBLE FOR IMPLEMENTING & MONITORING THE APPROVED EROSION & SEDIMENTATION CONTROL PLAN:

PAUL VIBORG

FIRE SAFETY PLAN

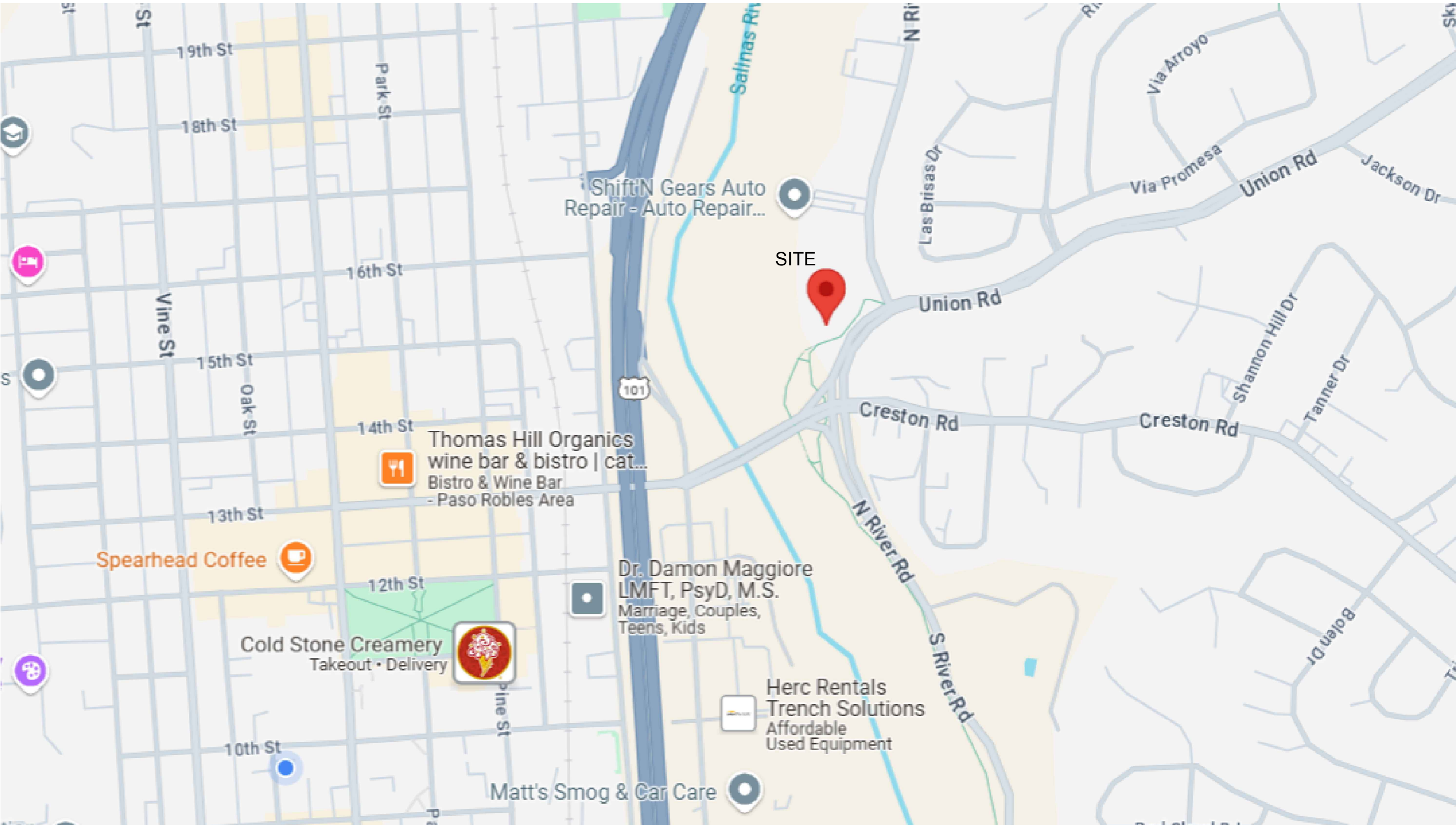
THE APPROVED PROJECT ALLOWED TO BE CONSTRUCTED BY THIS BUILDING PERMIT SHALL CONFORM TO THE FIRE SAFETY PLAN REQUIREMENTS AS DEEMED NECESSARY BY THE FIRE DEPARTMENT HAVING JURISDICTION FOR THIS PERMIT. PRIOR TO BEGINNING CONSTRUCTION THE PROPERTY OWNER SHALL READ THE FIRE SAFETY PLAN ISSUED BY THE FIRE DEPARTMENT AND BECOME FULLY AWARE OF ALL NECESSARY FIRE PROTECTION REQUIREMENTS.

1. WHEN FIRE SPRINKLERS ARE REQUIRED, A FIRE SPRINKLER PLAN AND PERMIT FOR RESIDENTIAL PROJECTS ARE REQUIRED. PROVIDE APPROVED PLANS TO BUILDING INSPECTOR PRIOR TO THE TIME OF FRAMING INSPECTION
2. PRIOR TO FINAL APPROVAL, THE PROPERTY SHALL BE IN COMPLIANCE WITH THE VEGETATION CLEARANCE. WHERE APPLICABLE, PROVIDE FIREBREAK WITHIN 30' AND 100' OF EACH BUILDING OR STRUCTURE. DOWNED LOGS, STUMPS, DEAD AND DYING WOODY SURFACE FUELS SHALL BE REMOVED. REMOVE SURFACE FUELS GREATER THAN 4 INCHES AND LOWER LIMBS OF TREES UP TO 6 FEET WITHIN AREAS OF CONTINUOUS TREE CANOPY

GENERAL GRADING NOTES

1. ANY AND ALL SITE WORK AND GRADING SHALL BE IN ACCORDANCE WITH CBC CHAPTER 18 AND CBC APPENDIX J AND ANY APPLICABLE LOCAL ORDINANCES.
2. A SOILS ENGINEER SHALL DETERMINE GRADING PERFORMED IS IN SUBSTANTIAL CONFORMANCE WITH THE APPROVED PLANS AND IS SUITABLE TO SUPPORT THE INTENDED STRUCTURE(S).
3. THE BOTTOM OF ALL EXCAVATIONS SHOULD BE OBSERVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PROCESSING OR PLACING FILL.
4. AN ENCROACHMENT PERMIT IS REQUIRED FOR ANY WORK DONE WITHIN A RIGHT OF WAY MAINTAINED BY THE PRESIDING JURISDICTION.
5. MAXIMUM CUT AND FILL SLOPE TO BE 2:1.
6. THE EXISTING GROUND SURFACE SHOULD BE PREPARED FOR GRADING BY REMOVING ALL VEGETATION, TREES, LARGE ROOTS, DEBRIS, NON-COMPLYING FILL, AND ALL OTHER ORGANIC MATERIAL. VOIDS CREATED BY REMOVAL OF SUCH MATERIALS SHOULD NOT BE BACKFILLED UNTIL THE UNDERLYING SOIL HAS BEEN OBSERVED BY A SOILS ENGINEER.
7. FILL AND BACKFILL SHOULD BE PLACED AT NEAR OPTIMUM MOISTURE IN LAYERS WITH LOOSE THICKNESS NOT GREATER THAN EIGHT (8) INCHES AND COMPACTED TO A MINIMUM OF 90% OF THE MAXIMUM DRY DENSITY OBTAINABLE BY TEST METHOD ASTM-D 1557, AND CERTIFIED BY A SOILS ENGINEER.
8. IMPORT SOILS USED TO RAISE SITE GRADE SHOULD BE EQUAL TO OR BETTER THAN ON-SITE SOILS IN STRENGTH, EXPANSION AND COMPRESSIBILITY CHARACTERISTICS. IMPORT SOIL CAN BE EVALUATED BUT WILL NOT BE PRE-QUALIFIED BY THE GEOTECHNICAL ENGINEER. FINAL COMMENTS ON THE CHARACTERISTICS OF THE IMPORT SOIL WILL BE PROVIDED AFTER THE MATERIAL IS STOCKPILED AT THE PROJECT SITE.
9. FINAL SITE GRADE SHOULD BE SUCH THAT ALL WATER IS DIVERTED AWAY FROM THE STRUCTURE(S) A MINIMUM OF 5% FOR 10 FEET. WATER SHALL NOT POND. ALL SURFACE WATER SHOULD BE DIRECTED INTO APPROVED DISCHARGE STRUCTURES.
10. ACCESS ROAD/DRIVEWAYS: ANY ROAD GRADE IN EXCESS OF 12% SHALL BE PAVED WITH A NON-SKID MATERIAL. GRADE FOR FIRE ACCESS SHALL NOT EXCEED 20%.
11. ALL NON-PERMITTED FILL SHALL BE REMOVED BY CONTRACTOR.
12. ELECTRICAL, TELECOMMUNICATIONS, AND OTHER UTILITIES SHALL BE INSTALLED UNDERGROUND IN AN APPROVED METHOD OF CONSTRUCTION. THIS REGULATION APPLIES TO UTILITIES ON SITES THAT ARE 5 ACRES OR LESS AND SERVING NEW STRUCTURES AND/OR NEW UTILITY DISTRIBUTIONS.
13. UTILITY TRENCH BACKFILL SHOULD BE GOVERNED BY THE PROVISIONS OF THIS REPORT RELATING TO MINIMUM COMPACTION STANDARDS. IN GENERAL, SERVICE LINES INSIDE THE PROPERTY LINES MAY BE BACKFILLED WITH NATIVE SOILS COMPACTED TO A MINIMUM OF 90% OF MAXIMUM DENSITY. BACKFILL OF OFF SITE SERVICE LINES WILL BE SUBJECT TO THE SPECIFICATIONS OF THE JURISDICTIONAL AGENCY OR THE GEOTECHNICAL REPORT, WHICHEVER IS GREATER.
14. LINED DRAINAGE SWALES AND DOWN DRAINS SHOULD BE PROVIDED AT THE TOPS OF CUT AND FILL SLOPES TO DIVERT DRAINAGE AWAY FROM SLOPE FACES.
15. FILL SLOPES SHOULD BE KEYED AND BENCHED INTO FIRM NATURAL GROUND WHEN THE EXISTING SLOPE TO RECEIVE FILL IS 5:1 OR STEEPER, HORIZONTAL TO VERTICAL. THE KEYS SHOULD BE TILTED INTO THE SLOPE A MINIMUM OF 2%, SHOULD BE A MINIMUM OF ONE EQUIPMENT WIDTH AND SHOULD BE A MINIMUM OF THREE (3) FEET DEEP ON THE OUTSIDE EDGE. ALL KEYS AND BENCHES SHOULD BE OBSERVED AND VERIFIED BY THE GEOTECHNICAL ENGINEER.

ARCHITECTURAL, CIVIL AND STRUCTURAL  
PLANS FOR A NEW OPEN SHED  
AT 1541 N. RIVER ROAD



VICINITY MAP

AIR QUALITY CONTROL

DURING CONSTRUCTION/GROUND DISTURBING ACTIVITIES, THE FOLLOWING PARTICULATE (DUST) CONTROL MEASURES SHALL BE IMPLEMENTED. THE CONTRACTOR OR BUILDER SHALL BE DESIGNATED TO MONITOR THE DUST CONTROL PROGRAM AND ORDER INCREASED WATERING, AS NECESSARY, TO PREVENT TRANSPORT OF DUST OFF SITE. THEIR DUTIES SHALL INCLUDE HOLIDAY AND WEEKEND PERIODS WHEN WORK MAY NOT BE IN PROGRESS. THEIR CONTACT INFORMATION SHALL BE PRESENTED TO THE APCD PRIOR TO COMMENCEMENT OF CONSTRUCTION.

1. REDUCE THE AMOUNT OF DISTURBED AREA WHERE POSSIBLE
2. USE OF WATER TRUCKS OR SPRINKLER SYSTEMS IN SUFFICIENT QUANTITIES TO PREVENT AIRBORNE DUST FROM LEAVING THE SITE. INCREASED WATERING FREQUENCY WOULD BE REQUIRED WHENEVER WIND SPEEDS EXCEED 15 MPH. RECLAIMED (NONPOTABLE) WATER SHOULD BE USED WHENEVER POSSIBLE;
3. ALL DIRT STOCK-PILE AREAS SHOULD BE SPRAYED DAILY AS NEEDED;
4. ALL ROADWAYS, DRIVEWAYS, SIDEWALKS, ETC TO BE PAVED SHALL BE COMPLETED AS SOON AS POSSIBLE; AND
5. BUILDING PADS SHALL BE LAID AS SOON AS POSSIBLE AFTER GRADING UNLESS SEEDING OR SOIL BINDERS ARE USED.

DURING INITIAL GRADING/SCRAPING, BURNING SHALL NOT BE ALLOWED, OR IF NO ALTERNATIVE IS AVAILABLE, THE APPLICANT SHALL OBTAIN A BURN PERMIT FROM THE APCD AND COUNTY FIRE/CALIFORNIA DEPARTMENT OF FORESTRY, AND COMPLY WITH ALL CONDITIONS REQUIRED BY THESE AGENCIES.

SEPARATE PERMIT REQUIREMENTS

FIRE SPRINKLERS

STATEMENT OF SPECIAL INSPECTIONS:

(22 CBC Section 1704.3):

SPECIAL INSPECTIONS ARE NOT REQUIRED FOR THIS PROJECT

PROJECT DESCRIPTION

THE PURPOSE OF THIS PLAN IS TO CONSTRUCT A NEW OPEN SHED METAL BUILDING FOR STORAGE OF YARD EQUIPMENT PER PLANS ATTACHED.

SHEET INDEX

T-1	TITLE SHEET, PROJECT INFORMATION
C-1	SITE PLAN AND GENERAL NOTES
C-2	EROSION CONTROL PLAN
C-3	GRADING PLAN
A-1.1	FLOOR PLAN
A-2.1	ELEVATIONS
A-2.2	ELEVATIONS
PS-1.1	PERSPECTIVE VIEW
GC-1.1	GREEN CODE SHEET 1
GC-1.2	GREEN CODE SHEET 2
ASP-1.1	ARCHITECTURAL NOTES AND SPECIFICATIONS
S-0	STRUCTURAL FOUNDATION TITLE SHEET
S-1	FOUNDATION PLAN
SN-1	FOUNDATION NOTES AND DETAILS
F1-F3	ANCHOR ROD SETTING PLANS AND DETAILS
E1-E12	METAL BUILDING PLANS

PROJECT INFORMATION

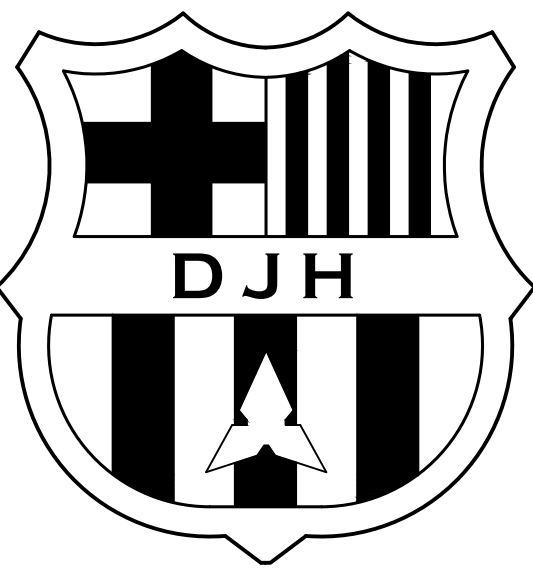
OWNER	PAUL VIBORG
PROJECT ADDRESS	1541 RIVER ROAD PASO ROBLES, CA 93446
APN	025-501-017
PHONE	805-674-9732

PROJECT STATISTICS

LOT SIZE	1.25 ACRES
OCCUPANCY (CBC 310.1)	U
CONSTRUCTION TYPE	VB
NEW OPEN SHED STORAGE BUILDING 4,080 SF	
FIRE SPRINKLERS	NO
WUI	NO

CONSULTANTS

STRUCTURAL ENGINEERING
DARRELL KUDLA 610 10TH ST., SUITE A PASO ROBLES, CA 93446
DESIGN AND DRAFTING
DH DRAFT AND DESIGN 610 10TH ST, SUITE A PASO ROBLES, CA 93446 805.975.3071
METAL BUILDING
STAR BUILDING SYSTEMS
ARCHITECT
KMN ARCHITECT KEN M. MACAHARA 610 10TH ST, SUITE A PASO ROBLES, CA 93446 805.610.7006



DRAFT & DESIGN

610 10TH ST. SUITE A  
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DJH1132@GMAIL.COM  
805.975.3071

PLAN PREPARED FOR:

PAUL VIBORG  
1541 N. RIVER ROAD  
PASO ROBLES, CA 93446



REVISION LOG

REV.	DESCRIPTION	DATE

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PROJECT NO. ---

FILE NAME T-1 TITLE SHEET.DWG

DRAWN BY MTS

DATE 2/24/2025 9:36 AM

SHEET TITLE:

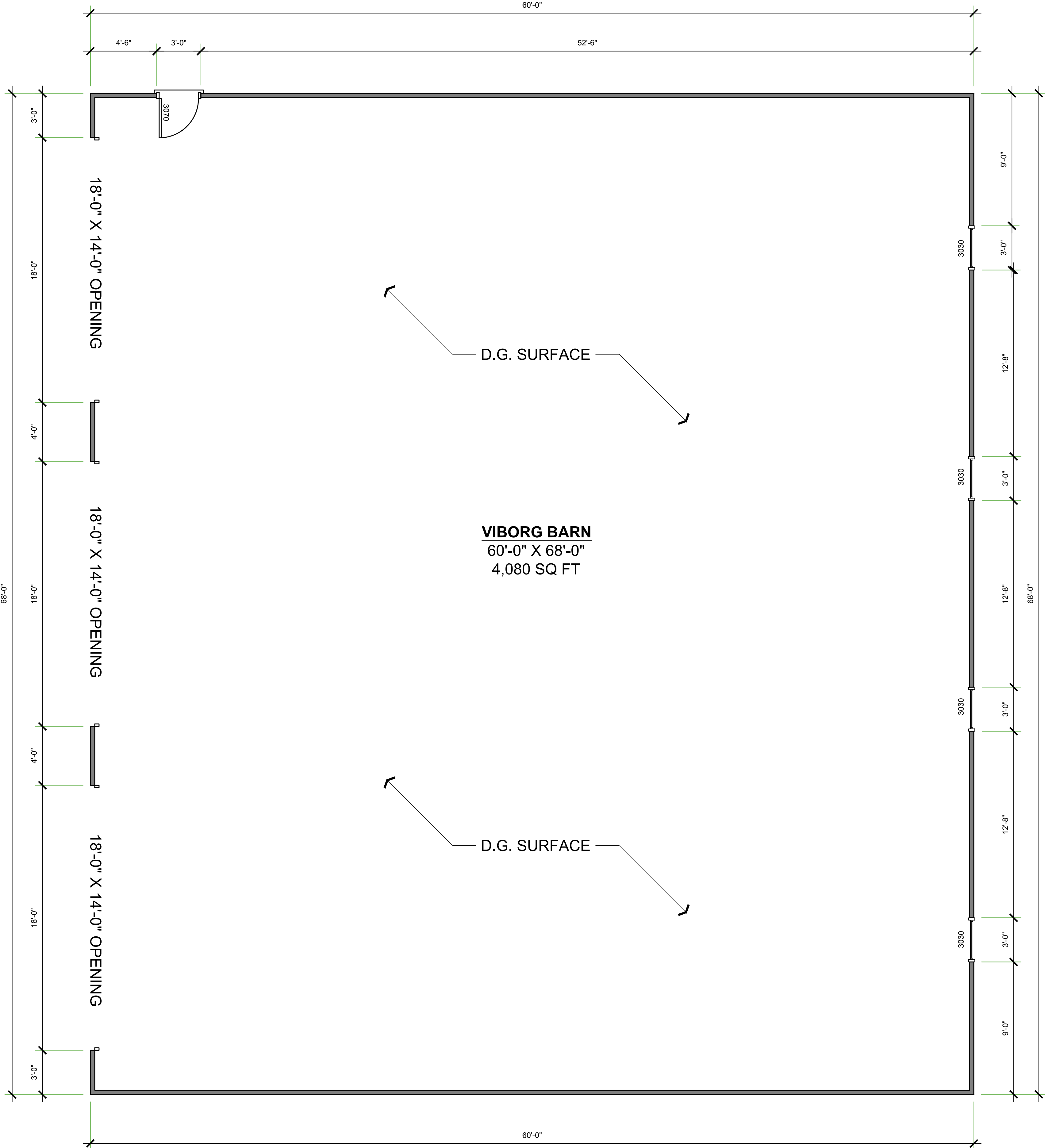
TITLE SHEET

SHEET NUMBER:

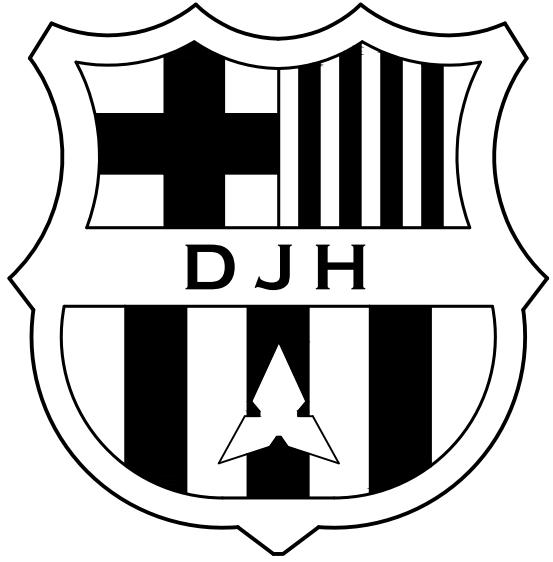
T-1.1



C:\Users\gh11\Desktop\projects\_2024\mborg Paul Personal Building\000000\project\Templates\SheetA-1-1 FLOOR PLAN.dwg Data Humphrey 2/24/2025 9:15:11 AM



**FLOOR PLAN**  
1/4" = 1'



**DRAFT & DESIGN**  
610 10TH ST. SUITE A  
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805.975.3071

PLAN PREPARED FOR:

**PAUL VIBORG**  
1541 NORTH RIVER ROAD  
PASO ROBLES, CA 93446



REVISION LOG		
REV.	DESCRIPTION	DATE

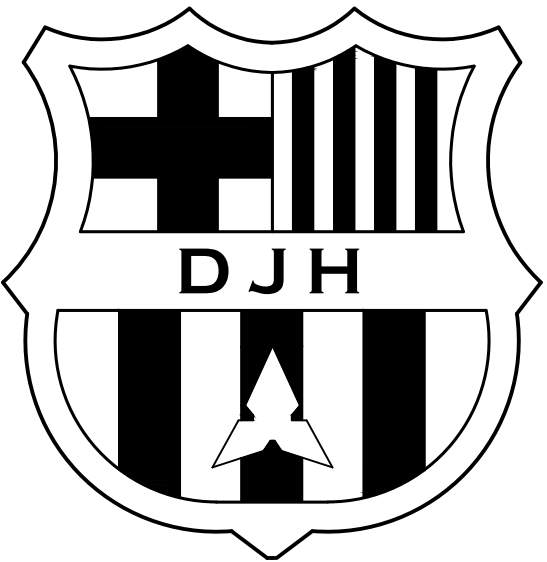
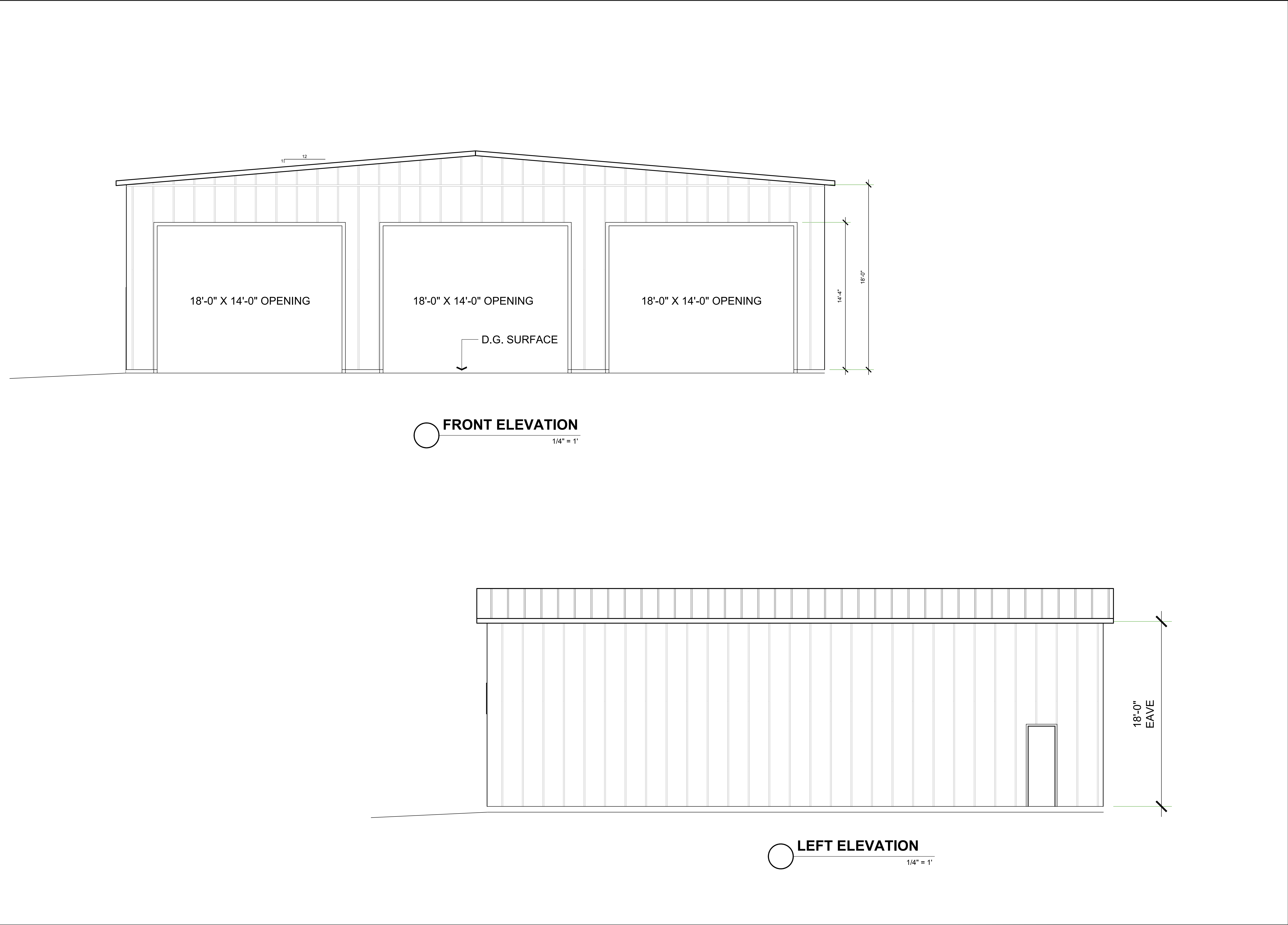
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PROJECT NO. ---  
FILE NAME A-1.1 FLOOR PLAN.DWG  
DRAWN BY MTS  
DATE 2/24/2025 9:15 AM  
SHEET TITLE:

**FLOOR PLAN**

SHEET NUMBER:

**A-1.1**



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610 10TH ST. SUITE A  
PASO ROBLES CA 93446  
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805.975.3071

PLAN PREPARED FOR:  
**PAUL VIBORG**  
**1541 NORTH RIVER ROAD**  
**PASO ROBLES, CA 93446**



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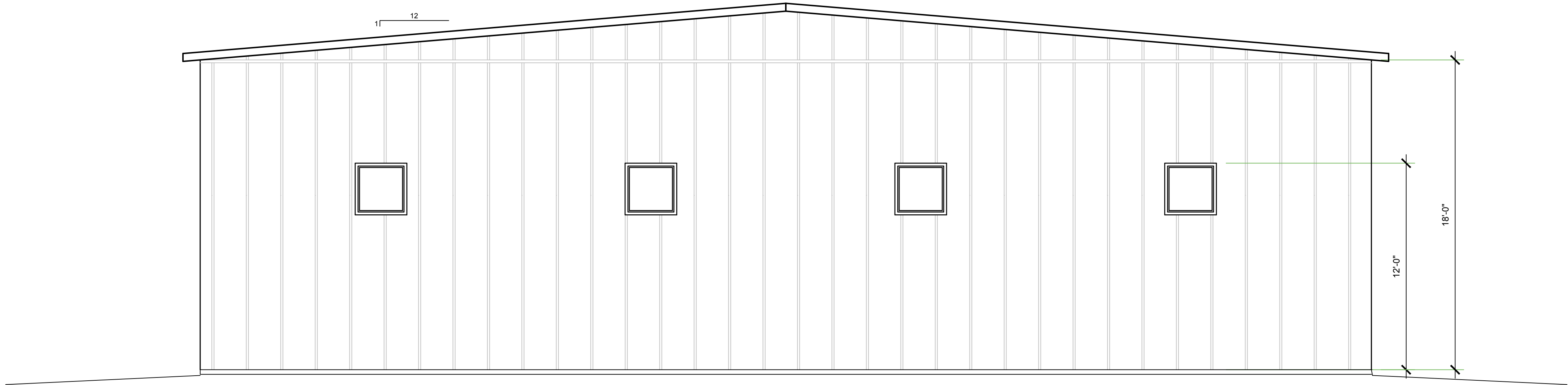
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DATE 2/24/2025 9:16 AM  
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**ELEVATIONS**

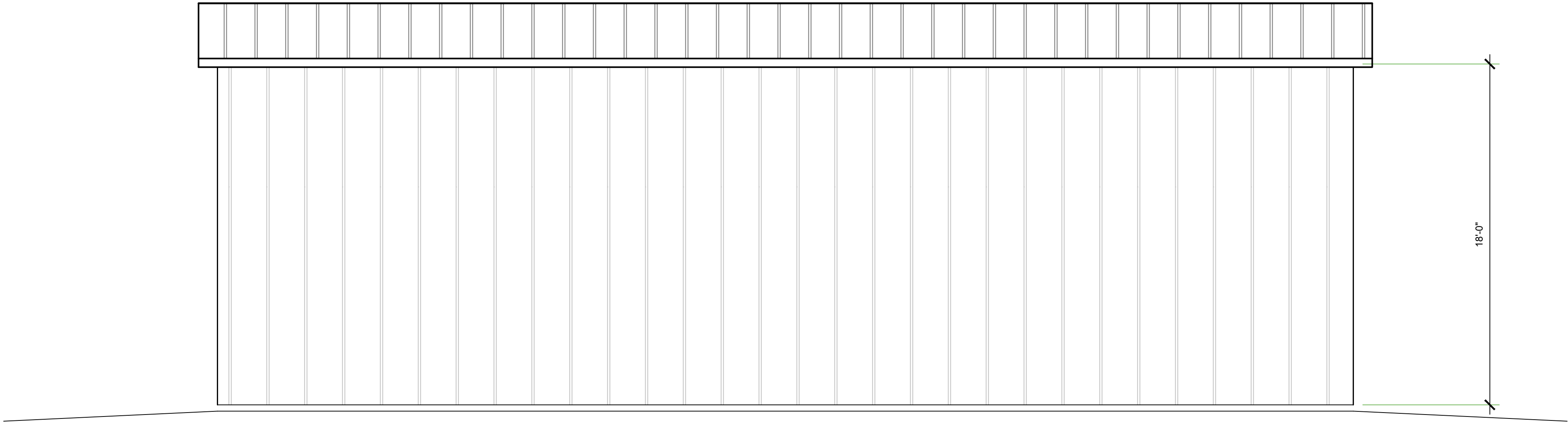
SHEET NUMBER:  
**A-2.1**



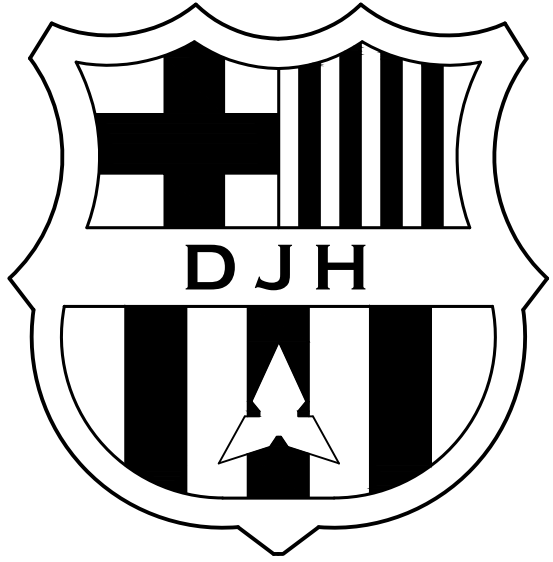
C:\Users\gh11\Desktop\projects\_2024\mborg Paul Personal Building\000000\project\Templates\SheetA-2-2 ELEVATIONS 2.dwg Dana Humphrey 2/24/2025 9:16:18 AM



BACK ELEVATION  
1/4" = 1'



RIGHT ELEVATION  
1/4" = 1'



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610 10TH ST. SUITE A  
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805.975.3071

PLAN PREPARED FOR:

PAUL VIBORG  
1541 NORTH RIVER ROAD  
PASO ROBLES, CA 93446



REVISION LOG

REV.	DESCRIPTION	DATE

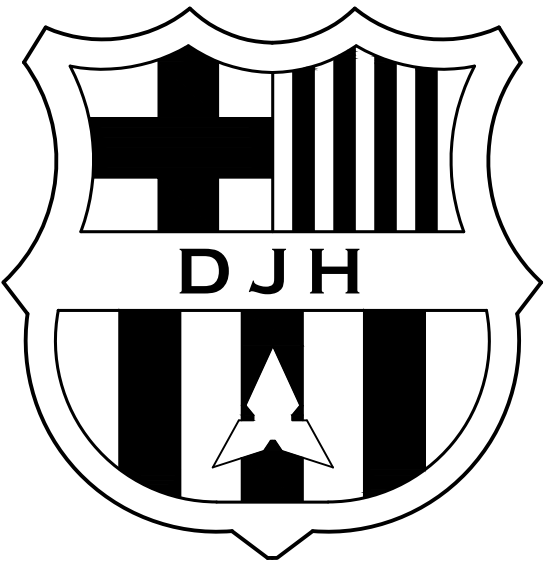
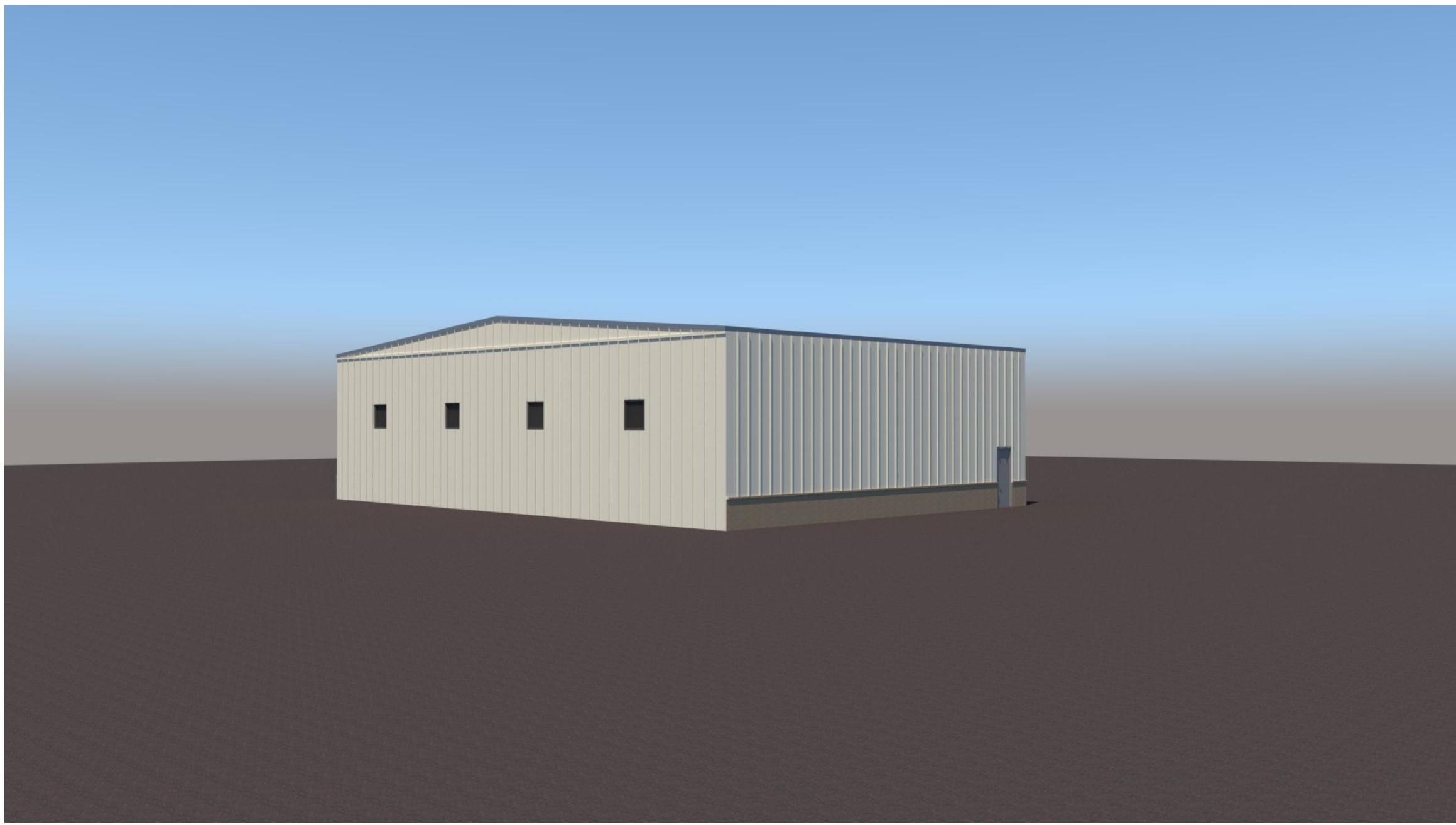
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PROJECT NO. ---  
FILE NAME A-2.2 ELEVATIONS 2.DWG  
DRAWN BY MTS  
DATE 2/24/2025 9:16 AM  
SHEET TITLE:

ELEVATIONS 2

SHEET NUMBER:

A-2.2



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PASO ROBLES CA 93446  
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PLAN PREPARED FOR:  
**PAUL VIBORG**  
**1541 NORTH RIVER ROAD**  
**PASO ROBLES, CA 93446**



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PROJECT NO. ---  
FILE NAME PS-1.1 PERSPECTIVE VIEW.DWG  
DRAWN BY MTS  
DATE 2/24/2025 9:16 AM

SHEET TITLE:  
**PERSPECTIVE VIEW**

SHEET NUMBER:  
**PS-1.1**



2022 CAL GREEN BUILDING STANDARD CODES: RESIDENTIAL MANDATORY MEASURES

CHAPTER 3  
GREEN BUILDING  
SECTION 301 GENERAL

**301.1 SCOPE.** Buildings shall be designed to include the green building measures specified as mandatory in the application checklists contained in this code. Voluntary green building measures are also included in the application checklists and may be included in the design and construction of structures covered by this code, but are not required unless adopted by a city, county, or city and county as specified in Section 101.7.

**301.1.1 Additions and alterations. [HCD]** The mandatory provisions of Chapter 4 shall be applied to additions or alterations of existing residential buildings where the addition or alteration increases the building's conditioned area, volume, or size. The requirements shall apply only to and/or within the specific area of the addition or alteration.

The mandatory provision of Section 4.106.4.2 may apply to additions or alterations of existing parking facilities or the addition of new parking facilities serving existing multifamily buildings. See Section 4.106.4.3 for application.

**Note:** Repairs including, but not limited to, resurfacing, restriping and repairing or maintaining existing lighting fixtures are not considered alterations for the purpose of this section.

**Note:** On and after January 1, 2014, residential buildings undergoing permitted alterations, additions, or improvements shall replace noncompliant plumbing fixtures with water-conserving plumbing fixtures. Plumbing fixture replacement is required prior to issuance of a certificate of final completion, certificate of occupancy or final permit approval by the local building department. See Civil Code Section 1101.1, et seq., for the definition of a noncompliant plumbing fixture, types of residential buildings affected and other important enactment dates.

**301.2 LOW-RISE AND HIGH-RISE RESIDENTIAL BUILDINGS. [HCD]** The provisions of individual sections of CALGreen may apply to either low-rise residential buildings high-rise residential buildings, or both. Individual sections will be designated by banners to indicate where the section applies specifically to low-rise only (LR) or high-rise only (HR). When the section applies to both low-rise and high-rise buildings, no banner will be used.

SECTION 302 MIXED OCCUPANCY BUILDINGS

**302.1 MIXED OCCUPANCY BUILDINGS.** In mixed occupancy buildings, each portion of a building shall comply with the specific green building measures applicable to each specific occupancy.

- Exceptions:
1. [HCD] Accessory structures and accessory occupancies serving residential buildings shall comply with Chapter 4 and Appendix A4, as applicable.
  2. [HCD] For purposes of CALGreen, live/work units, complying with Section 419 of the *California Building Code*, shall not be considered mixed occupancies. Live/Work units shall comply with Chapter 4 and Appendix A4, as applicable.

DIVISION 4.1 PLANNING AND DESIGN

ABBREVIATION DEFINITIONS:

HCD	Department of Housing and Community Development
BSC	California Building Standards Commission
DSA-SS	Division of the State Architect, Structural Safety
OSH/PD	Office of Statewide Health Planning and Development
LR	Low Rise
HR	High Rise
AA	Additions and Alterations
N	New

CHAPTER 4  
RESIDENTIAL MANDATORY MEASURES

SECTION 4.102 DEFINITIONS

**4.102.1 DEFINITIONS**  
The following terms are defined in Chapter 2 (*and are included here for reference*)

**FRENCH DRAIN.** A trench, hole or other depressed area loosely filled with rock, gravel, fragments of brick or similar pervious material used to collect or channel drainage or runoff water.

**WATTLES.** Wattles are used to reduce sediment in runoff. Wattles are often constructed of natural plant materials such as hay, straw or similar material shaped in the form of tubes and placed on a downflow slope. Wattles are also used for perimeter and inlet controls.

4.106 SITE DEVELOPMENT

**4.106.1 GENERAL.** Preservation and use of available natural resources shall be accomplished through evaluation and careful planning to minimize negative effects on the site and adjacent areas. Preservation of slopes, management of storm water drainage and erosion controls shall comply with this section.

**4.106.2 STORM WATER DRAINAGE AND RETENTION DURING CONSTRUCTION.** Projects which disturb less than one acre of soil and are not part of a larger common plan of development which in total disturbs one acre or more, shall manage storm water drainage during construction. In order to manage storm water drainage during construction, one or more of the following measures shall be implemented to prevent flooding of adjacent property, prevent erosion and retain soil runoff on the site.

1. Retention basins of sufficient size shall be utilized to retain storm water on the site.
2. Where storm water is conveyed to a public drainage system, collection point, gutter or similar disposal method, water shall be filtered by use of a barrier system, wattle or other method approved by the enforcing agency.
3. Compliance with a lawfully enacted storm water management ordinance.

**Note:** Refer to the State Water Resources Control Board for projects which disturb one acre or more of soil, or are part of a larger common plan of development which in total disturbs one acre or more of soil.

(Website: [https://www.waterboards.ca.gov/water\\_issues/programs/stormwater/construction.html](https://www.waterboards.ca.gov/water_issues/programs/stormwater/construction.html))

**4.106.3 GRADING AND PAVING.** Construction plans shall indicate how the site grading or drainage system will manage all surface water flows to keep water from entering buildings. Examples of methods to manage surface water include, but are not limited to, the following:

1. Swales
2. Water collection and disposal systems
3. French drains
4. Water retention gardens
5. Other water measures which keep surface water away from buildings and aid in groundwater recharge.

**Exception:** Additions and alterations not altering the drainage path.

**4.106.4 Electric vehicle (EV) charging for new construction.** New construction shall comply with Sections 4.106.4.1 or 4.106.4.2 to facilitate future installation and use of EV chargers. Electric vehicle supply equipment (EVSE) shall be installed in accordance with the *California Electrical Code*, Article 625.

- Exceptions:
1. On a case-by-case basis, where the local enforcing agency has determined EV charging and infrastructure are not feasible based upon one or more of the following conditions:
    - 1.1 Where there is no local utility power supply or the local utility is unable to supply adequate power.
    - 1.2 Where there is evidence suitable to the local enforcing agency substantiating that additional local utility infrastructure design requirements, directly related to the implementation of Section 4.106.4, may adversely impact the construction cost of the project.
  2. Accessory Dwelling Units (ADU) and Junior Accessory Dwelling Units (JADU) without additional parking facilities.

**4.106.4.1 New one- and two-family dwellings and townhouses with attached private garages.** For each dwelling unit, install a listed raceway to accommodate a dedicated 208/240-volt branch circuit. The raceway shall not be less than trade size 1 (nominal 1-inch inside diameter). The raceway shall originate at the main service or subpanel and shall terminate into a listed cabinet, box or other enclosure in close proximity to the proposed location of an EV charger. Raceways are required to be continuous at enclosed, inaccessible or concealed areas and spaces. The service panel and/or subpanel shall provide capacity to install a 40-ampere 208/240-volt minimum dedicated branch circuit and space(s) reserved to permit installation of a branch circuit overcurrent protective device.

Exemption: A raceway is not required if a minimum 40-ampere 208/240-volt dedicated EV branch circuit is installed in close proximity to the proposed location of an EV charger at the time of original construction in accordance with the *California Electrical Code*.

**4.106.4.1.1 Identification.** The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging as "EV CAPABLE". The raceway termination location shall be permanently and visibly marked as "EV CAPABLE".

**4.106.4.2 New multifamily dwellings, hotels and motels and new residential parking facilities.** When parking is provided, parking spaces for new multifamily dwellings, hotels and motels shall meet the requirements of Sections 4.106.4.2.1 and 4.106.4.2.2. Calculations for spaces shall be rounded up to the nearest whole number. A parking space served by electric vehicle supply equipment or designed as a future EV charging space shall count as at least one standard automobile parking space only for the purpose of complying with any applicable minimum parking space requirements established by a local jurisdiction. See Vehicle Code Section 22511.2 for further details.

**4.106.4.2.1 Multifamily development projects with less than 20 dwelling units; and hotels and motels with less than 20 sleeping units or guest rooms.** The number of dwelling units, sleeping units or guest rooms shall be based on all buildings on a project site subject to this section.

**1.EV Capable.** Ten (10) percent of the total number of parking spaces on a building site, provided for all types of parking facilities, shall be electric vehicle charging spaces (EV spaces) capable of supporting future Level 2 EVSE. Electrical load calculations shall demonstrate that the electrical panel service capacity and electrical system, including any on-site distribution transformer(s), have sufficient capacity to simultaneously charge all EVs at all required EV spaces at a minimum of 40 amperes.

The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging purposes as "EV CAPABLE" in accordance with the California Electrical Code.

Exceptions:

- 1.When EV chargers (Level 2 EVSE) are installed in a number equal to or greater than the required number of EV capable spaces.
- 2.When EV chargers (Level 2 EVSE) are installed in a number less than the required number of EV capable spaces, the number of EV capable spaces required may be reduced by a number equal to the number of EV chargers installed.

Notes:

a.Construction documents are intended to demonstrate the project's capability and capacity for facilitating future EV charging.

b.There is no requirement for EV spaces to be constructed or available until receptacles for EV charging or EV chargers are installed for use.

**2.EV Ready.** Twenty-five (25) percent of the total number of parking spaces shall be equipped with low power Level 2 EV charging receptacles. For multifamily parking facilities, no more than one receptacle is required per dwelling unit when more than one parking space is provided for use by a single dwelling unit.

Exception: Areas of parking facilities served by parking lifts.

**4.106.4.2.2 Multifamily development projects with 20 or more dwelling units, hotels and motels with 20 or more sleeping units or guest rooms.** The number of dwelling units, sleeping units or guest rooms shall be based on all buildings on a project site subject to this section.

**1.EV Capable.** Ten (10) percent of the total number of parking spaces on a building site, provided for all types of parking facilities, shall be electric vehicle charging spaces (EV spaces) capable of supporting future Level 2 EVSE. Electrical load calculations shall demonstrate that the electrical panel service capacity and electrical system, including any on-site distribution transformer(s), have sufficient capacity to simultaneously charge all EVs at all required EV spaces at a minimum of 40 amperes.

The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging purposes as "EV CAPABLE" in accordance with the California Electrical Code.

Exception: When EV chargers (Level 2 EVSE) are installed in a number greater than five (5) percent of parking spaces required by Section 4.106.4.2.2, Item 3, the number of EV capable spaces required may be reduced by a number equal to the number of EV chargers installed over the five (5) percent required.

Notes:

a.Construction documents shall show locations of future EV spaces.

b.There is no requirement for EV spaces to be constructed or available until receptacles for EV charging or EV chargers are installed for use.

**2.EV Ready.** Twenty-five (25) percent of the total number of parking spaces shall be equipped with low power Level 2 EV charging receptacles. For multifamily parking facilities, no more than one receptacle is required per dwelling unit when more than one parking space is provided for use by a single dwelling unit.

Exception: Areas of parking facilities served by parking lifts.

**3.EV Chargers.** Five (5) percent of the total number of parking spaces shall be equipped with Level 2 EVSE. Where common use parking is provided, at least one EV charger shall be located in the common use parking area and shall be available for use by all residents or guests.

When low power Level 2 EV charging receptacles or Level 2 EVSE are installed beyond the minimum required, an automatic load management system (ALMS) may be used to reduce the maximum required electrical capacity to each space served by the ALMS. The electrical system and any on-site distribution transformers shall have sufficient capacity to deliver at least 3.3 kW simultaneously to each EV charging station (EVCS) served by the ALMS. The branch circuit shall have a minimum capacity of 40 amperes, and installed EVSE shall have a capacity of not less than 30 amperes. ALMS shall not be used to reduce the minimum required electrical capacity to the required EV capable spaces.

**4.106.4.2.2.1 Electric vehicle charging stations (EVCS).** Electric vehicle charging stations required by Section 4.106.4.2.2, Item 3, shall comply with Section 4.106.4.2.2.1.

Exception: Electric vehicle charging stations serving public accommodations, public housing, motels and hotels shall not be required to comply with this section. See California Building Code, Chapter 11B, for applicable requirements.

**4.106.4.2.2.1.1 Location.** EVCS shall comply with at least one of the following options:

- 1.The charging space shall be located adjacent to an accessible parking space meeting the requirements of the California Building Code, Chapter 11A, to allow use of the EV charger from the accessible parking space.
- 2.The charging space shall be located on an accessible route, as defined in the California Building Code, Chapter 2, to the building.

Exception: Electric vehicle charging stations designed and constructed in compliance with the California Building Code, Chapter 11B, are not required to comply with Section 4.106.4.2.2.1.1 and Section 4.106.4.2.2.1.2, Item 3.

**4.106.4.2.2.1.2 Electric vehicle charging stations (EVCS) dimensions.** The charging spaces shall be designed to comply with the following:

- 1.The minimum length of each EV space shall be 18 feet (5486 mm).
- 2.The minimum width of each EV space shall be 9 feet (2743 mm).
- 3.One in every 25 charging spaces, but not less than one, shall also have an 8-foot (2438 mm) wide minimum aisle. A 5-foot (1524 mm) wide minimum aisle shall be permitted provided the minimum width of the EV space is 12 feet (3658 mm).

a.Surface slope for this EV space and the aisle shall not exceed 1 unit vertical in 48 units horizontal (2.083 percent slope) in any direction.

**4.106.4.2.2.1.3 Accessible EV spaces.** In addition to the requirements in Sections 4.106.4.2.2.1.1 and 4.106.4.2.2.1.2, all EVSE, when installed, shall comply with the accessibility provisions for EV chargers in the California Building Code, Chapter 11B. EV ready spaces and EVCS in multifamily developments shall comply with California Building Code, Chapter 11A, Section 1109A.

**4.106.4.2.3 EV space requirements.**  
1.Single EV space required. Install a listed raceway capable of accommodating a 208/240-volt dedicated branch circuit. The raceway shall not be less than trade size 1 (nominal 1-inch inside diameter). The raceway shall originate at the main service or subpanel and shall terminate into a listed cabinet, box or enclosure in close proximity to the location or the proposed location of the EV space. Construction documents shall identify the raceway termination point, receptacle or charger location, as applicable. The service panel and/or subpanel shall have a 40-ampere minimum dedicated branch circuit, including branch circuit overcurrent protective device installed, or space(s) reserved to permit installation of a branch circuit overcurrent protective device.

Exception: A raceway is not required if a minimum 40-ampere 208/240-volt dedicated EV branch circuit is installed in close proximity to the location or the proposed location of the EV space, at the time of original construction in accordance with the California Electrical Code.

2.Multiple EV spaces required. Construction documents shall indicate the raceway termination point and the location of installed or future EV spaces, receptacles or EV chargers. Construction documents shall also provide information on amperage of installed or future receptacles or EVSE, raceway method(s), wiring schematics and electrical load calculations. Plan design shall be based upon a 40-ampere minimum branch circuit. Required raceways and related components that are planned to be installed underground, enclosed, inaccessible or in concealed areas and spaces shall be installed at the time of original construction.

Exception: A raceway is not required if a minimum 40-ampere 208/240-volt dedicated EV branch circuit is installed in close proximity to the location or the proposed location of the EV space at the time of original construction in accordance with the California Electrical Code.

**4.106.4.2.4 Identification.** The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging purposes as "EV CAPABLE" in accordance with the California Electrical Code.

**4.106.4.2.5 Electric Vehicle Ready Space Signage.** Electric vehicle ready spaces shall be identified by signage or pavement markings, in compliance with Caltrans Traffic Operations Policy Directive 13-01 (Zero Emission Vehicle Signs and Pavement Markings) or its successor(s).

**4.106.4.3 Electric vehicle charging for additions and alterations of parking facilities serving existing multifamily buildings.** When new parking facilities are added, or electrical systems or lighting of existing parking facilities are added or altered and the work requires a building permit, ten (10) percent of the total number of parking spaces added or altered shall be electric vehicle charging spaces (EV spaces) capable of supporting future Level 2 EVSE.

Notes:

1.Construction documents are intended to demonstrate the project's capability and capacity for facilitating future EV charging.

2.There is no requirement for EV spaces to be constructed or available until EV chargers are installed for use.

DIVISION 4.2 ENERGY EFFICIENCY

4.201 GENERAL

**4.201.1 SCOPE.** For the purposes of mandatory energy efficiency standards in this code, the California Energy Commission will continue to adopt mandatory standards.

DIVISION 4.3 WATER EFFICIENCY AND CONSERVATION

4.303 INDOOR WATER USE

**4.303.1 WATER CONSERVING PLUMBING FIXTURES AND FITTINGS.** Plumbing fixtures (water closets and urinals) and fittings (faucets and showerheads) shall comply with the sections 4.303.1.1, 4.303.1.2, 4.303.1.3, and 4.303.4.4.

**Note:** All noncompliant plumbing fixtures in any residential real property shall be replaced with water-conserving plumbing fixtures. Plumbing fixture replacement is required prior to issuance of a certificate of final completion, certificate of occupancy, or final permit approval by the local building department. See Civil Code Section 1101.1, et seq., for the definition of a noncompliant plumbing fixture, types of residential buildings affected and other important enactment dates.

**4.303.1.1 Water Closets.** The effective flush volume of all water closets shall not exceed 1.28 gallons per flush. Tank-type water closets shall be certified to the performance criteria of the U.S. EPA WaterSense Specification for Tank-type Toilets.

**Note:** The effective flush volume of dual flush toilets is defined as the composite, average flush volume of two reduced flushes and one full flush.

**4.303.1.2 Urinals.** The effective flush volume of wall mounted urinals shall not exceed 0.125 gallons per flush. The effective flush volume of all other urinals shall not exceed 0.5 gallons per flush.

**4.303.1.3 Showerheads.**

**4.303.1.3.1 Single Showerhead.** Showerheads shall have a maximum flow rate of not more than 1.8 gallons per minute at 80 psi. Showerheads shall be certified to the performance criteria of the U.S. EPA WaterSense Specification for Showerheads.

**4.303.1.3.2 Multiple showerheads serving one shower.** When a shower is served by more than one showerhead, the combined flow rate of all the showerheads and/or other shower outlets controlled by a single valve shall not exceed 1.8 gallons per minute at 80 psi, or the shower shall be designed to only allow one shower outlet to be in operation at a time.

**Note:** A hand-held shower shall be considered a showerhead.

**4.303.1.4 Faucets.**

**4.303.1.4.1 Residential Lavatory Faucets.** The maximum flow rate of residential lavatory faucets shall not exceed 1.2 gallons per minute at 60 psi. The minimum flow rate of residential lavatory faucets shall not be less than 0.8 gallons per minute at 20 psi.

**4.303.1.4.2 Lavatory Faucets in Common and Public Use Areas.** The maximum flow rate of lavatory faucets installed in common and public use areas (outside of dwellings or sleeping units) in residential buildings shall not exceed 0.5 gallons per minute at 60 psi.

**4.303.1.4.3 Metering Faucets.** Metering faucets when installed in residential buildings shall not deliver more than 0.2 gallons per cycle.

**4.303.1.4.4 Kitchen Faucets.** The maximum flow rate of kitchen faucets shall not exceed 1.8 gallons per minute at 60 psi. Kitchen faucets may temporarily increase the flow above the maximum rate, but not to exceed 2.2 gallons per minute at 60 psi, and must default to a maximum flow rate of 1.8 gallons per minute at 60 psi.

**Note:** Where complying faucets are unavailable, aerators or other means may be used to achieve reduction.

**4.303.1.4.5 Pre-rinse spray valves.** When installed, shall meet the requirements in the *California Code of Regulations*, Title 20 (Appliance Efficiency Regulations), Sections 1605.1 (h)(4) Table H-2, Section 1605.3 (h)(4)(A), and Section 1607 (d)(7) and shall be equipped with an integral automatic shutoff.

**FOR REFERENCE ONLY:** The following table and code section have been reprinted from the *California Code of Regulations*, Title 20 (Appliance Efficiency Regulations),Section 1605.1 (h)(4) and Section 1605.3 (h)(4)(A).

TABLE H-2		
STANDARDS FOR COMMERCIAL PRE-RINSE SPRAY VALVES MANUFACTURED ON OR AFTER JANUARY 28, 2019		
PRODUCT CLASS [spray force in ounce force (ozf)]	MAXIMUM FLOW RATE (gpm)	
Product Class 1 (≤ 5.0 ozf)	1.00	
Product Class 2 (> 5.0 ozf and ≤ 8.0 ozf)	1.20	
Product Class 3 (> 8.0 ozf)	1.28	

Title 20 Section 1605.3 (h)(4)(A): Commercial prerinse spray valves manufactured on or after January 1, 2006, shall have a minimum spray force of not less than 4.0 ounces-force (ozf)(113 grams-force)(gf)

**4.303.2 Submeters for multifamily buildings and dwelling units in mixed-used residential/commercial buildings.**

Submeters shall be installed to measure water usage of individual rental dwelling units in accordance with the *California Plumbing Code*.

**4.303.3 Standards for plumbing fixtures and fittings.** Plumbing fixtures and fittings shall be installed in accordance with the *California Plumbing Code*, and shall meet the applicable standards referenced in Table 1701.1 of the *California Plumbing Code*.

TABLE - MAXIMUM FIXTURE WATER USE		
FIXTURE TYPE	FLOW RATE	
SHOWER HEADS (RESIDENTIAL)	1.8 GMP @ 80 PSI	
LAVATORY FAUCETS (RESIDENTIAL)	MAX. 1.2 GPM @ 60 PSI MIN. 0.8 GMP @ 20 PSI	
LAVATORY FAUCETS IN COMMON & PUBLIC USE AREAS	0.5 GPM @ 60 PSI	
KITCHEN FAUCETS	1.8 GPM @ 60 PSI	
METERING FAUCETS	0.2 GAL/CYCLE	
WATER CLOSET	1.28 GAL/FLUSH	
URINALS	0.125 GAL/FLUSH	

4.304 OUTDOOR WATER USE

**4.304.1 OUTDOOR POTABLE WATER USE IN LANDSCAPE AREAS.** Residential developments shall comply with a local water efficient landscape ordinance or the current California Department of Water Resources' Model Water Efficient Landscape Ordinance (MWELO), whichever is more stringent.

NOTES:

1. The Model Water Efficient Landscape Ordinance (MWELO) is located in the *California Code Regulations*, Title 23, Chapter 2.7, Division 2. MWELO and supporting documents, including water budget calculator, are available at: <https://www.water.ca.gov/>

DIVISION 4.4 MATERIAL CONSERVATION AND RESOURCE EFFICIENCY

4.406 ENHANCED DURABILITY AND REDUCED MAINTENANCE

**4.406.1 RODENT PROOFING.** Annular spaces around pipes, electric cables, conduits or other openings in sole/bottom plates at exterior walls shall be protected against the passage of rodents by closing such openings with cement mortar, concrete masonry or a similar method acceptable to the enforcing agency.

4.408 CONSTRUCTION WASTE REDUCTION, DISPOSAL AND RECYCLING

**4.408.1 CONSTRUCTION WASTE MANAGEMENT.** Recycle and/or salvage for reuse a minimum of 75 percent of the non-hazardous construction and demolition waste in accordance with either Section 4.408.2, 4.408.3 or 4.408.4, or meet a more stringent local construction and demolition waste management ordinance.

Exceptions:

1. Excavated soil and land-clearing debris.
2. Alternate waste reduction methods developed by working with local agencies if diversion or recycle facilities capable of compliance with this item do not exist or are not located reasonably close to the jobsite.
3. The enforcing agency may make exceptions to the requirements of this section when isolated jobsites are located in areas beyond the haul boundaries of the diversion facility.

**4.408.2 CONSTRUCTION WASTE MANAGEMENT PLAN.** Submit a construction waste management plan in conformance with Items 1 through 5. The construction waste management plan shall be updated as necessary and shall be available during construction for examination by the enforcing agency.

1. Identify the construction and demolition waste materials to be diverted from disposal by recycling, reuse on the project or salvage for future use or sale.
2. Specify if construction and demolition waste materials will be sorted on-site (source separated) or bulk mixed (single stream).
3. Identify diversion facilities where the construction and demolition waste material collected will be taken.
4. Identify construction methods employed to reduce the amount of construction and demolition waste generated.
5. Specify that the amount of construction and demolition waste materials diverted shall be calculated by weight or volume, but not by both.

**4.408.3 WASTE MANAGEMENT COMPANY.** Utilize a waste management company, approved by the enforcing agency, which can provide verifiable documentation that the percentage of construction and demolition waste material diverted from the landfill complies with Section 4.408.1.

**Note:** The owner or contractor may make the determination if the construction and demolition waste materials will be diverted by a waste management company.

**4.408.4 WASTE STREAM REDUCTION ALTERNATIVE [LR].** Projects that generate a total combined weight of construction and demolition waste disposed of in landfills, which do not exceed 3.4 lbs./sq.ft. of the building area shall meet the minimum 65% construction waste reduction requirement in Section 4.408.1

**4.408.4.1 WASTE STREAM REDUCTION ALTERNATIVE.** Projects that generate a total combined weight of construction and demolition waste disposed of in landfills, which do not exceed 2 pounds per square foot of the building area, shall meet the minimum 65% construction waste reduction requirement in Section 4.408.1

**4.408.5 DOCUMENTATION.** Documentation shall be provided to the enforcing agency which demonstrates compliance with Section 4.408.2, Items 1 through 5, Section 4.408.3 or Section 4.408.4..

Notes:

1. Sample forms found in "A Guide to the California Green Building Standards Code (Residential)" located at [www.hcd.ca.gov/CALGreen.html](http://www.hcd.ca.gov/CALGreen.html) may be used to assist in documenting compliance with this section.
2. Mixed construction and demolition debris (C & D) processors can be located at the California Department of Resources Recycling and Recovery (CalRecycle).

4.410 BUILDING MAINTENANCE AND OPERATION

**4.410.1 OPERATION AND MAINTENANCE MANUAL.** At the time of final inspection, a manual, compact disc, web-based reference or other media acceptable to the enforcing agency which includes all of the following shall be placed in the building:

1. Directions to the owner or occupant that the manual shall remain with the building throughout the life cycle of the structure.
2. Operation and maintenance instructions for the following:
  - a. Equipment and appliances, including water-saving devices and systems, HVAC systems, photovoltaic systems, electric vehicle chargers, water-heating systems and other major appliances and equipment.
  - b. Roof and yard drainage, including gutters and downspouts.
  - c. Space conditioning systems, including condensers and air filters.
  - d. Landscape irrigation systems.
  - e. Water reuse systems.
3. Information from local utility, water and waste recovery providers on methods to further reduce resource consumption, including recycle programs and locations.
4. Public transportation and/or carpool options available in the area.
5. Educational material on the positive impacts of an interior relative humidity between 30-60 percent and what methods an occupant may use to maintain the relative humidity level in that range.
6. Information about water-conserving landscape and irrigation design and controllers which conserve water.
7. Instructions for maintaining gutters and downspouts and the importance of diverting water at least 5 feet away from the foundation.
8. Information on required routine maintenance measures, including, but not limited to, caulking, painting, grading around the building, etc.
9. Information about state solar energy and incentive programs available.
10. A copy of all special inspections verifications required by the enforcing agency or this code.
11. Information from the Department of Forestry and Fire Protection on maintenance of defensible space around residential structures.
12. Information and/or drawings identifying the location of grab bar reinforcements.

**4.410.2 RECYCLING BY OCCUPANTS.** Where 5 or more multifamily dwelling units are constructed on a building site, provide readily accessible area(s) that serves all buildings on the site and are identified for the depositing, storage and collection of non-hazardous materials for recycling, including (at a minimum) paper, corrugated cardboard, glass, plastics, organic waste, and metals, or meet a lawfully enacted local recycling ordinance, if more restrictive.

**Exception:** Rural jurisdictions that meet and apply for the exemption in Public Resources Code Section 42649.82 (a)(2)(A) et seq.



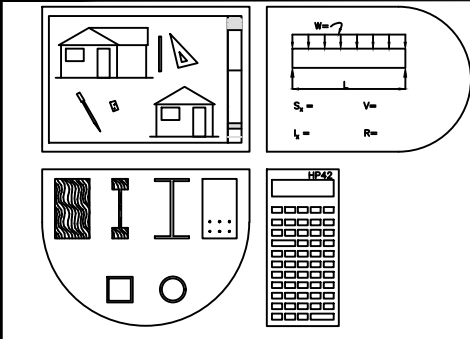




## ARCHITECTURAL NOTES AND SPECIFICATIONS

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UNIQUE PERSPECTIVES  
ARCHITECTURAL ENGINEERING

STRUCTURAL  
CONSULTANT

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PLAN PREPARED FOR:  
  
PAUL VIBORG  
1541 N. RIVER RD  
PASO ROBLES, CA

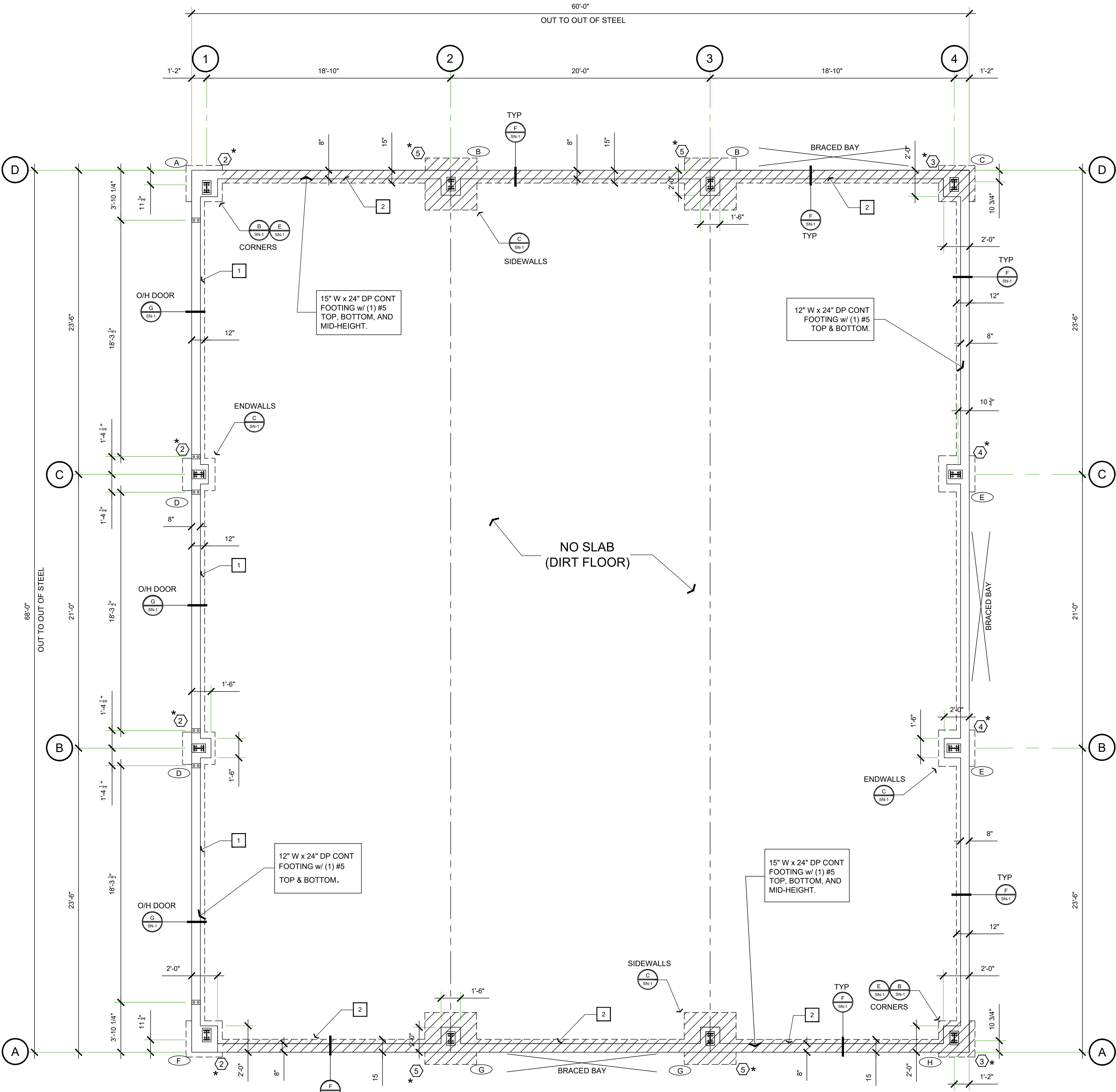
# METAL BUILDING FOUNDATION *DESIGN* for:

## VIBORG SAND AND GRAVEL 1529 N. RIVER RD PASO ROBLES, CA

ABBREVIATIONS:

#	NUMBER OR POUND	FAB	FABRICATED	PROJ	PROJECT, PROJECTS, PROJECTION
~	APPROXIMATELY	FDN	FOUNDATION	PSF	POUNDS PER SQUARE FOOT
<	ANGLE	FSI	FINISH GRADE	PSI	POUNDS PER SQUARE INCH
@	AT	FG	FINISH GRADE	PSL	PARALLEL LUMBER
AB	ANCHOR BOLT	FJ	FLOOR JOIST	PT	PRESSURE TREATED
ABV	ABOVE	FLR	FLOOR	PTF	PRESSURE TREATED DOUGLAS FIR
ADDL	ADDITIONAL	FN	FIELD NAIL	PLY	PLYWOOD
ADJ	ADJACENT	FOC	FACE OF CONCRETE	QTY	QUANTITY
AIRC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION	FON	FACE OF MASONRY		
AFF	ALTERNATE	FOS	FACE OF STUD		
ALT	ALTERNATE	FRMG	ABOVE FINISH FLOOR		
APA	AMERICAN PLYWOOD ASSOCIATION	FT	FOOT, FEET		
APPROX	APPROXIMATE	FTG	FOOTING		
ARCH	ARCHITECTURAL ARCHITECT				
ASTM	AMERICAN STANDARDS OF TESTING AND MATERIALS	GA	GAUGE	R	RADIUS
BLDG	BUILDING	GALV	GALVANIZED	REIN	REINFORCING
BLK	BLOCK, BLOCKING	GLB	GLUE, LAMINATED BEAM	REQ	REQUIRED
BLW	BELOW	GYP	GYPSTRUM BOARD	RET	RETAINING
BM	BEAM	HD	HOLDOWN	REV	REVERSE (MIRROR)
BN	BOUNDARY NAILING	HG	HOT DIPPED GALVANIZED	SCHD	SCHEDULE
BOB	BOTTOM OF BEAM	HDR	HEADER	SECT	SECTION
BOT	BOTTOM	HOR	HANGER	SEL	SINGLE
BRG	BEARING	HSS	HORIZONTAL	SHT	SHEET
BTWN	BETWEEN	HT	HOLLOW STRUCTURAL SECTION	SHTG	SHEATHING
			HEIGHT	SH	SIMILAR
				SMS	SHEET METAL SCREW
				SOD	SLAB ON GRADE
				SPEC	SPECIFICATION
				SQ	SQUARE
				SS	STAINLESS STEEL
				STD	STANDARD
				STGD	STAGGERED
				STIFF	STIFFENER
				STL	STEEL
				STRUCT	STRUCTURAL
				STRGR	STRINGER
				SW	SHEAR WALL
				SYM	SYMMETRICAL





1 FOUNDATION PLAN  
1/4" = 1'

PAD FOOTING SCHEDULE			
CALL OUT	PAD SIZE	BOLT EMBED	REINFORCING
1	2'-0" SQ. x 24" DEEP	18"	(2) #4 EACH WAY TOP & BOTTOM
2	2'-6" SQ. x 24" DEEP	18"	(3) #4 EACH WAY TOP & BOTTOM
3	3'-0" SQ. x 36" DEEP	18"	(4) #4 EACH WAY TOP & BOTTOM
4	3'-6" SQ. x 36" DEEP	18"	(4) #4 EACH WAY TOP & BOTTOM
5	4'-0" SQ. x 36" DEEP	18"	(5) #4 EACH WAY TOP & BOTTOM
6	4'-6" SQ. x 21" DEEP		(5) #4 EACH WAY TOP & BOTTOM
7	5'-0" SQ. x 21" DEEP		(6) #4 EACH WAY TOP & BOTTOM

- #\* = DENOTES FOOTING WHICH REQUIRE STRUCTURAL OBSERVATIONS ANCHOR BOLTS, REINFORCING, AND HAIRPIN PLACEMENT.
- \* = WHEN FOOTING DEPTH EXCEED, OR SOILS ENGR REQUIRES FOOTINGS DEEPER THAN 24" BELOW GRADE: ADD (1) #5 VERTICAL BAR @ EACH CORNER OF FOOTING (4 TOTAL).
- \* = SEE PAD FOOTINGS DETAILS FOR EMBEDMET MEASUREMENT AND ANCHOR BOLT SPECIFICATIONS.

- METAL BUILDING FOUNDATION NOTES:**
- GENERAL:**
- VERIFY ALL DIMENSIONS AGAINST ARCHITECTURAL AND METAL BUILDING DRAWINGS.
  - Contractor to verify all foundation dimensions with metal building manufacturer's anchor bolt setting plan, report any differences to ENGINEER.
  - Details #'s A/- thru H/- are general foundation related conditions that are to be incorporated as they relate to applicable conditions and/or other details, even though they may not be specifically delineated on these plans.
  - NOT USED.
  - NOT USED.
  - All continuous rebar at exterior and interior footings which pass through a pad shall extend 4 feet beyond said pad without splices (min.) as per detail E/-.
  - All rebar bends to be made as per detail H/-.
  - Anchor bolt size, thread, count, location and projections are as per metal building manufacturer's anchor bolt setting plan. Minimum embedment depth of bolts at frames are to be 18" into pad footings (embedment depth is measured from the bottom of slab), see detail D/-; Anchor bolts to have 3", 90 degree hooks or headed studs with double nuts and 3" square x .25" thick A-36 steel plate washers.
  - See metal building plans for base plate details, special edge conditions and anchor bolt placements (use template method).
  - #\* indicates pad footing. See pad footing schedule and detail A/-.
  - E\* indicates base plate reference number. See anchor bolt placement plans (by metal building manufacturer) for proper locations and call-outs to be verified with this plan prior to construction by the contractor. Notify ENGINEER if discrepancies arise.
  - #\* indicates building/frame lines.
  - C\* indicates thrust hairpins. Thrust Hairpins shall be #4 with " crown and " long legs (minimum unless otherwise noted). Locate thrust hairpins around each anchor bolt pair with 2" clear from top of slab. See plans for locations.

**FOUNDATION DESIGN SOIL VALUES:**

BEARING =1000 PSF, FROM REPORT  
EXP. INDEX= MED. FROM REPORT  
\*NO ON-SITE INVESTIGATION PERFORMED BY STRUCT. ENGR

REFER TO THE "GENERAL SPECIFICATIONS FOR SOILS, FOUNDATIONS, CONCRETE AND REINFORCING" NOTES ON SN-1 FOR ADDITIONAL SPECIFICATIONS TO BE PART OF THE FOUNDATION CONSTRUCTION PACKAGE.

**SOILS REPORT REQUIREMENTS:**

REFER TO THE PROJECT SOILS REPORT BY HALLEN GEOTECHNICAL, FILE # H-81134 DATED 7-16-18, FOR SOILS CONDITIONS AND SITE PREPARATION REQUIREMENTS. REPORT SHALL BE CONSIDERED PART OF THIS FOUNDATION PLAN AND ALL RECOMMENDATIONS SHALL BE IMPLEMENTED ACCORDINGLY.

**BUILDING PAD, FOUNDATION AND SLAB PREP:**

BUILDING PAD PREPARATION REQUIREMENTS SHALL BE PER PROJECT SOILS REPORT, WHERE REQUIRED, OVER EXCAVATION AND SCARIFICATION SHOULD EXTEND AT LEAST 5' BEYOND THE PERIMETER OF THE BUILDING LINES.

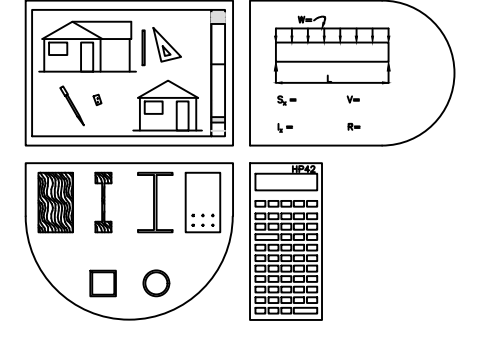
THE DEPTH OF BUILDING FOOTINGS SHALL BE DETERMINED ON-SITE BY SOILS ENGINEER. DEPTH OF FOOTINGS SHALL BEGIN AT COMPETENT MATERIAL WHICH MAY NOT BE THE SAM AS FINISHED GRADE. THE FOUNDATION EXCAVATIONS SHALL BE OBSERVED BY THE ENGINEERING GEOLOGIST PRIOR TO ANY REINFORCING OR CONCRETE IS PLACED. FOR FOOTINGS DEEPER THAN NOTED ON THESE PLANS AFTER EXCAVATIONS, CONTACT FOUNDATION ENGINEER (TRAVERSO) FOR DEEPEENED FOOTING REINFORCING REQUIREMENTS.

SLABS ON GRADE SUB-BASE PREPARATION AND MOISTURE CONTROL REQUIREMENTS SHALL BE AS REQUIRED BY SOILS REPORT. SOIL ENGINEER SHALL INSPECT AND APPROVE MOISTURE CONDITIONS OF ALL UNDER-SLAB AREAS JUST PRIOR TO CONCRETE PLACEMENT.

**SOILS CONDITION OBSERVATIONS & NOTIFICATION:**

PRIOR TO THE CONTRACTOR REQUESTING A BUILDING DEPARTMENT FOUNDATION INSPECTION, THE SOILS ENGINEER SHALL ADVISE THE BUILDING OFFICIAL IN WRITING THAT: (1) THE BUILDING PAD WAS PREPARED IN ACCORDANCE WITH THE SOILS REPORT, (2) THE UTILITY TRENCHES HAVE BEEN PROPERLY BACKFILLED AND COMPACTED, (3) THE FOUNDATION EXCAVATIONS COMPLY WITH THE INTENT OF THE SOILS REPORT.

- KEYENOTES:**
- 1 = REDUCE CURB HEIGHT AT OVERHEAD DOOR AS REQUIRED SEE DETAIL G-SN-1.
  - 2 = CROSS HATCHED AREAS INDICATE 36" DEEP FOOTINGS



UNIQUE PERSPECTIVES  
ARCHITECTURAL ENGINEERING

STRUCTURAL  
CONSULTANT

DARIN TRAVERSO, R.C.E.  
Phone: (805)434-2950  
Fax: (805)434-2909

1155 Rolfe In.  
Templeton, CA 93465



PLAN PREPARED FOR:

PAUL VIBORG  
1541 N. RIVER RD  
PASO ROBLES, CA

REVISION LOG		
REV.	DESCRIPTION	DATE

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PROJECT NO.  
FILE NAME MTL BLDG.DWG  
DRAWN BY AT  
DATE 2/21/2025 8:25 PM

SHEET TITLE:  
FOUNDATION  
PLAN

SHEET NUMBER:  
S-1



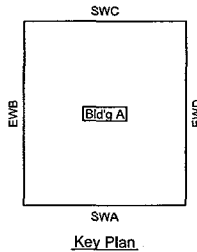
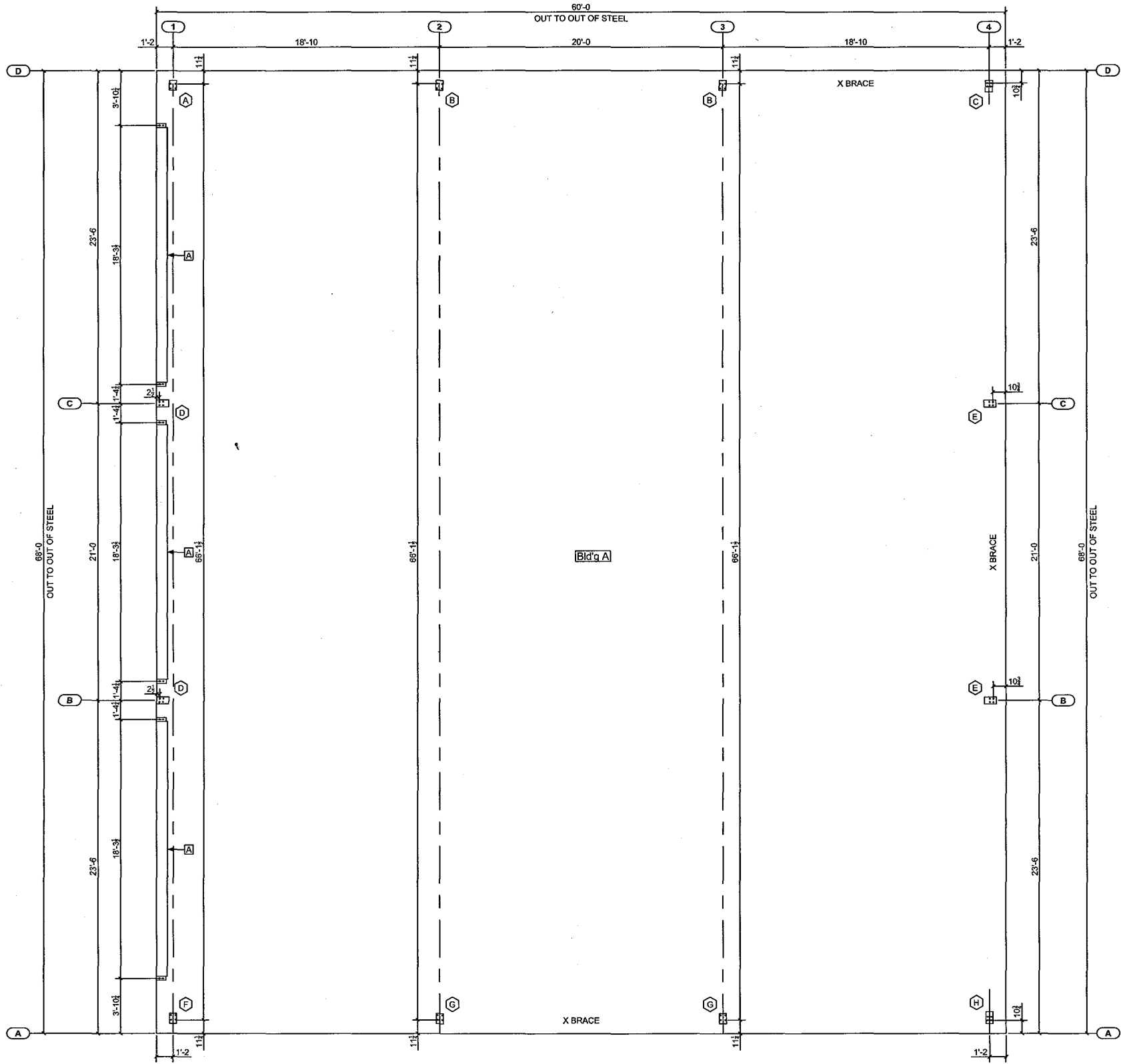




Anchor Rod Drawings

- 1) This drawing is for anchor rod placement only and is not foundation design.
- 2) Foundation must be square and level with all anchor rods true in size, location, and projection.
- 3) Projection shown must be held to keep threads clear of finished concrete.
- 4) This structural design data includes magnitude and location of design loads and support conditions, material properties, and type and size of major structural members necessary to show compliance with the Order Documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be superseded and voided by any future mailing.
- 5) Anchor rod size as noted on the drawings has been determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer. Anchor rods are not provided by the metal building manufacturer.
- 6) Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise.
- 7) 3000 psi concrete compressive strength (f'c) is assumed for the purpose of column base plate design unless otherwise noted.

Finished Floor at Elevation 100'-0"



FRAMED OPENING SCHEDULE			
MARK	DESCRIPTION	DETAIL	QUAN.
1	18'-0" X 15'-0" FRAMED OPENING	1	3

Anchor Rod Setting Plan

DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.  
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE  
PERMIT DRAWINGS.

Revision	Date	Description	By	Ch'd

Cornerstone Building Brands  
13105 Northwest Freeway, Suite 500  
Houston, TX 77040  
cornerstonebuildingbrands.com

**STAR**  
Part of the Cornerstone Building Brands Family

Project Name & Location:  
PAUL VIBORG  
1528 N RIVER RD  
PASO ROBLES, CA 93446-7325

Customer:  
DUECK CONSTRUCTION COMPANY INC  
2313 SIGNORA ROSA CT  
PASO ROBLES, CA 93446  
ATTN: TIM DUECK

☐ Issued For Construction

☐ Issued For Approval  
☒ Issued For Permit

Scale: NOT TO SCALE

Drawn by: HPR 1/6/25

Checked by: HPR 1/6/25

Project Engineer: BXT

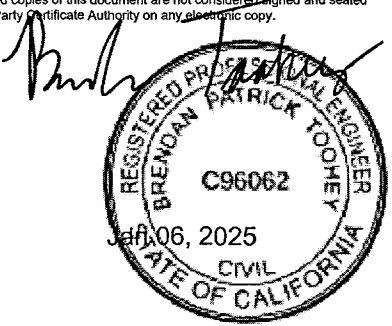
Job Number: 19-B-82019

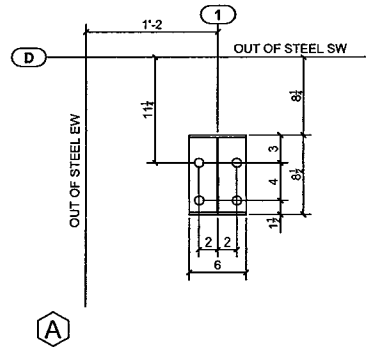
Sheet Number: F1 of 3

The engineer whose seal appears hereon is employed by or is contracted to provide engineering services for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

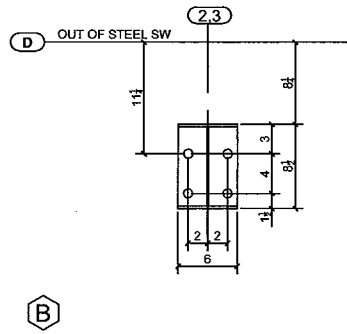
BRENDAN PATRICK TOOHEY, P.E.  
CALIFORNIA P.E. C96062

This item has been electronically signed and sealed by Brendan Patrick Toohey, P.E. on the date and/or time stamp shown using a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified by a 3rd Party Certificate Authority on any electronic copy.

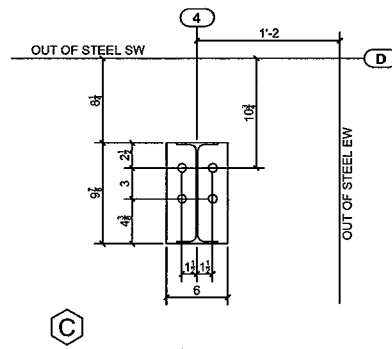




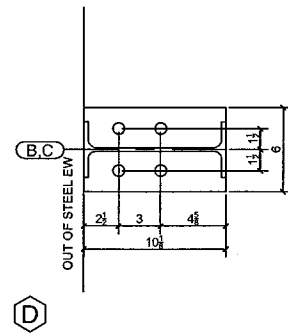
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ANCHOR ROD SIZE = 3/4" DIA.  
ANCHOR ROD PROJECTION = 3"  
BOTTOM OF BASE PLATE ELEVATION = 100'-0"



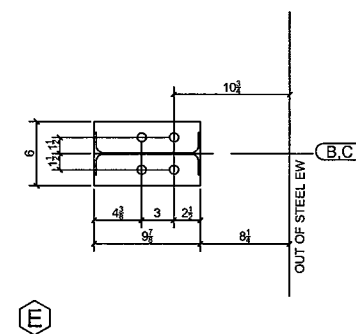
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ANCHOR ROD SIZE = 3/8" DIA.  
ANCHOR ROD PROJECTION = 3"  
BOTTOM OF BASE PLATE ELEVATION = 100'-0"



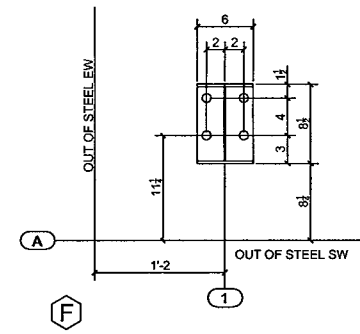
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ANCHOR ROD SIZE = 5/8" DIA.  
ANCHOR ROD PROJECTION = 2"  
BOTTOM OF BASE PLATE ELEVATION = 100'-0"



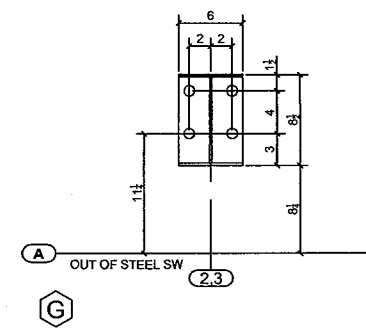
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ANCHOR ROD PROJECTION = 2"  
BOTTOM OF BASE PLATE ELEVATION = 100'-0"



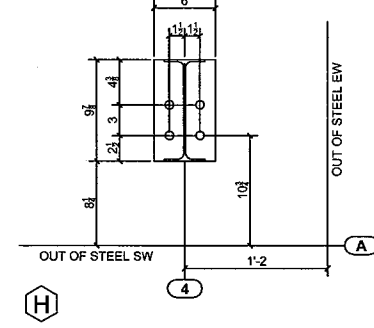
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ANCHOR ROD SIZE = 3/8" DIA.  
ANCHOR ROD PROJECTION = 2"  
BOTTOM OF BASE PLATE ELEVATION = 100'-0"



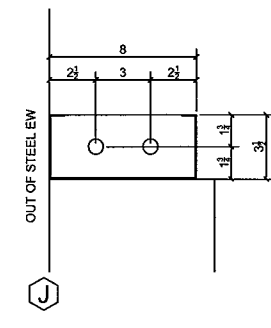
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ANCHOR ROD SIZE = 3/4" DIA.  
ANCHOR ROD PROJECTION = 3"  
BOTTOM OF BASE PLATE ELEVATION = 100'-0"



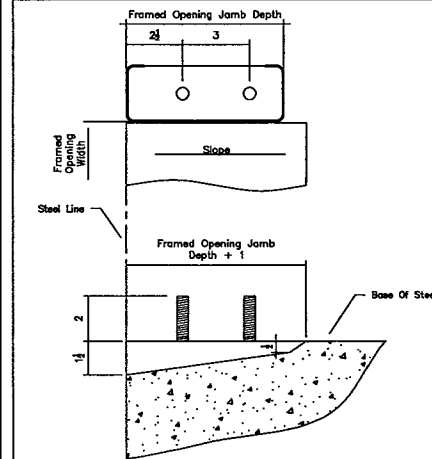
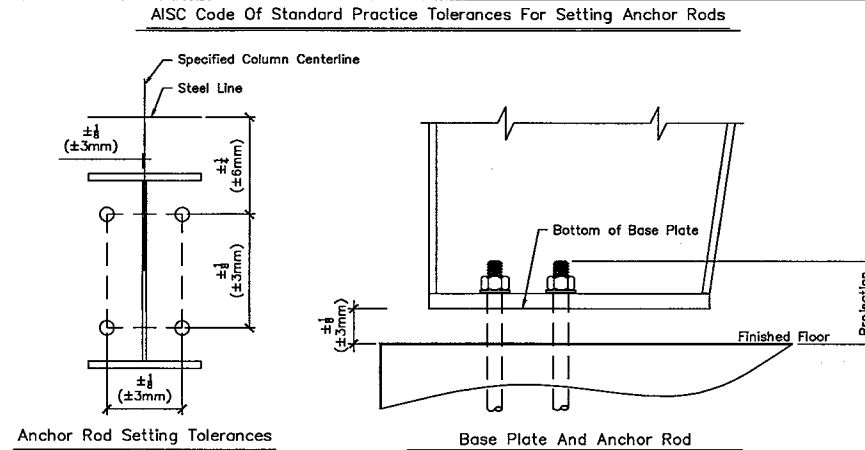
BASE PLATE SIZE = 6"W x 8 1/2"L x 3/8" THICK  
ANCHOR ROD SIZE = 3/4" DIA.  
ANCHOR ROD PROJECTION = 3"  
BOTTOM OF BASE PLATE ELEVATION = 100'-0"




BASE PLATE SIZE = 6"W x 9 $\frac{7}{8}$ "L x  $\frac{3}{8}$ " THICK  
ANCHOR ROD SIZE =  $\frac{3}{8}$ " DIA.  
ANCHOR ROD PROJECTION = 2"  
BOTTOM OF BASE PLATE ELEVATION = 100'-0"



BASE PLATE SIZE =  $3\frac{1}{2}"$  W x  $8"$  L x  $\frac{1}{4}"$  THICK  
ANCHOR ROD SIZE =  $\frac{5}{8}"$  DIA.  
ANCHOR ROD PROJECTION =  $2"$   
BOTTOM OF BASE PLATE ELEVATION =  $100'-0"$

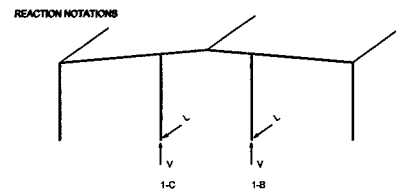


<div><p><b>STAR</b></p><p><small>Professional Seal Required Approved For Unpermitted Building Activity Only</small></p></div> <div><b>Customer:</b> DUECK CONSTRUCTION COMPANY INC 2315 SIGNORA ROSA CT PASO ROBLES, CA 93446 ATTN: TIM DUECK</div> <div><b>Project Name &amp; Location:</b> PAUL VIEORG 1529 N RIVER RD PASO ROBLES, CA 93446-7325 ATTN: TIM DUECK</div> <div><b>Drawing Status:</b> <input type="checkbox"/> Issued For Approval <input checked="" type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Not For Construction <input type="checkbox"/> Issued For Permit</div>	<b>Revision</b>	<b>Date</b>	<b>Description</b>	<b>By</b>	<b>Ck'd</b>	
<div><div>Cornerstone Building Brands 13105 Northwest Freeway, Suite 200, Houston, TX 77040 cornerstonebuildingbrands.com</div></div>						
<b>Scale:</b> NOT TO SCALE						
<b>Drawn by:</b> HPR 1/6/25						
<b>Checked by:</b> HPR 1/6/25						
<b>Project Engineer:</b> BXT						
<b>Job Number:</b> 19-B-82019						
<b>Sheet Number:</b> F2 of 3						
<p>The engineer whose seal appears herein is employed by or is contracted to provide engineering services for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.</p> <p>BRENDAN PATRICK TOOHEY, P.E. CALIFORNIA P.E. C96062</p>						

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DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.  
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE  
PERMIT DRAWINGS.

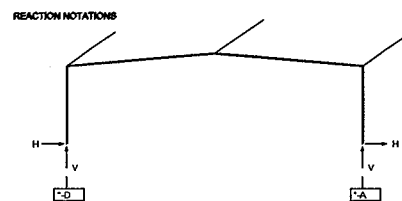
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SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.					TIME: 16:38:51



COLUMN LOAD GROUP	1-C			1-B		
	H	V	L	H	V	L
D	0.	1.4	0.0	0.	1.4	0.0
C	0.	1.2	0.0	0.	1.2	0.0
L	0.	4.7	-0.1	0.	4.7	-0.1
W-	0.	-5.0	-3.2	0.	-5.0	-3.2
W16+	0.	0.	3.4	0.	0.	3.4
W+	0.	-5.0	3.5	0.	-5.0	3.5
E+	0.	1.7	0.2	0.	1.7	0.3
E-	0.	-1.7	-0.2	0.	-1.7	-0.3

LOAD GROUP DESCRIPTION	
D	: Dead load
C	: Collateral load
L	: Live load
W-	: Wind load as an outward acting suction
W16+	: Min. 16 psf wind as an inward acting pressure
W+	: Wind load as an inward acting pressure
E+	: Seismic force acting inward
E-	: Seismic force acting outward

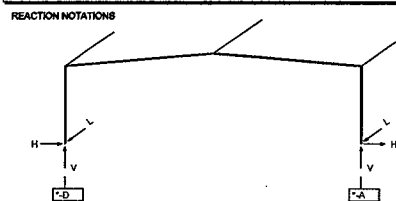
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SUPPORT REACTIONS FOR EACH LOAD GROUP			
*LOCATION: Girdlines: 1			
NOTES: (1) All reactions are in kips and kip-ft.			
			TIME: 11:50:10



COLUMN	+D			+A		
LOAD GROUP	H	V	L	H	V	L
DL	0.7	1.6	-0.0	-0.7	1.6	-0.0
LL	2.4	4.3	-0.0	-2.4	4.3	-0.0
COLL	1.0	1.8	-0.0	-1.0	1.8	-0.0
EQ	-1.0	-0.5	-0.0	1.0	0.0	-0.0
WL1	-4.7	-7.2	-0.0	4.7	-4.7	-0.0
WL2	-4.1	-5.0	-0.0	1.0	-2.5	-0.0
LWL1	-2.2	-6.7	-0.0	2.4	-5.2	-0.0
LWL2	-2.4	-5.2	-0.0	2.2	-6.7	-0.0
LWL3	-1.5	-4.5	-0.0	1.8	-3.0	-0.0
LWL4	-0.6	-3.0	-0.0	0.6	-4.5	-0.0
WL4	-1.7	-4.1	-0.0	4.7	-7.2	-0.0
LWL4	-1.0	-2.5	-0.0	4.1	-5.0	-0.0

LOAD GROUP DESCRIPTION	
DL	: Roof Dead Load
LL	: Roof Live Load
COLL	: Roof Collateral Load
EQ	: Lateral Seismic Load (parallel to plane of frame)
WL1	: Wind from Left to Right with +Gcpi
WL2	: Wind from Left to Right with -Gcpi
LWL1	: Windward Corner Left with +Gcpi
LWL2	: Windward Corner Right with -Gcpi
LWL3	: Windward Corner Left with -Gcpi
LWL4	: Windward Corner Right with +Gcpi
WL3	: Wind from Right to Left with +Gcpi
WL4	: Wind from Right to Left with -Gcpi

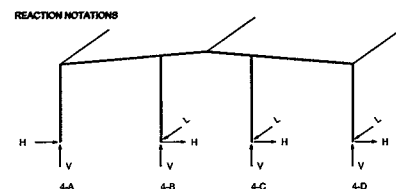
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SUPPORT REACTIONS FOR EACH LOAD GROUP			
*LOCATION: Gridline: 2 3			
NOTES: (1) All reactions are in kips and ft.			
(2) The seismic overstrength factor (Omega) is not included in the "RBDYEQ" and "RBLPEQ" Load Group reactions.			
*Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.000			
*Primary and load cases are not grouped.			
(3) X-bracing reactions (RBPULW and RBUPCO) are combined with LWL and LEQ groups only.			



LOAD GROUP REACTION TABLE GRIDLINES =							2	3
COLUMN	"Q"			"A"				
LOAD GROUP	H	V	L	H	V	L		
DL	1.3	2.3	-0.0	-1.3	2.3	-0.0		
LL	5.2	7.9	-0.0	-5.2	7.9	-0.0		
COLL	2.2	3.3	-0.0	-2.2	3.3	-0.0		
RBDWEQ	0.0	5.6	-0.0	0.0	5.3	-0.0		
EQ	-1.7	-0.8	-0.0	-1.7	-0.8	-0.0		
W1	0.0	-2.5	-0.0	0.0	-3.3	-0.0		
WL1	-6.7	-9.3	-0.0	3.0	-6.5	-0.0		
WL2	-5.2	5.3	-0.0	1.4	-2.5	-0.0		
WL3	-3.0	-4.5	-0.0	6.7	-9.3	-0.0		
WL4	-1.4	-2.5	-0.0	3.0	-6.5	-0.0		
LW1	-3.2	-6.8	-0.0	3.5	-7.1	-0.0		
RBLUFL	0.0	-6.1	-5.3	-0.0	-4.8	-5.3		
LW2	-3.5	-7.1	-0.0	3.2	-6.8	-0.0		
LW3	-1.6	-4.7	-0.0	1.8	-3.1	-0.0		
LW4	-1.9	-2.4	-0.0	1.9	-3.1	-0.0		
RBDWL	0.0	-5.1	-0.0	0.0	-4.5	-0.0		

LOAD GROUP DESCRIPTION	
DL	Roof Dead Load
LL	Roof Live Load
COLL	Roof Collocated Load
DRBQWG	Downward Acting Roof Braced Load from Long. Seismic
EB	East/East-Northeast Load parallel to view of from Long. Seismic
RBUPQF	Upward Acting Roof Braced Load from Long. Seismic
WL1	Wind from Left to Right with +GCP
WL2	Wind from Left to Right with +GCP
WL3	Wind from Right to Left with +GCP
WL4	Wind from Right to Left with -GCP
LWL1	Windward Corner Load with +GCP
LRBUP1W	Upward Acting Roof Braced Load from Long. Wind
LWL2	Windward Corner Load with +GCP
LWL3	Windward Corner Load with -GCP
LWL4	Windward Corner Load with -GCP
DRBWLW	Downward Acting Roof Braced Load from Long. Wind

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SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.					
TIME: 16:30:51					



LOAD GROUP REACTION TABLE												
COLUMN	4-A			4-B			4-C			4-D		
LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L
D	0.0	0.5	0.	0.	1.0	0.0	0.	1.0	0.0	0.0	0.5	0.
C	0.0	0.5	0.	0.	1.3	0.0	0.	1.3	0.0	0.0	0.5	0.
L	0.0	2.1	0.	0.	4.8	0.0	0.	4.8	0.0	0.0	2.1	0.
W+	-0.1	-3.3	0.	0.	-7.5	3.5	0.	-7.5	3.5	0.1	-4.3	5.3
W-	-0.1	-3.3	0.	0.	-7.5	-3.3	0.	-7.5	-3.3	0.1	1.8	0.
WRR	-0.1	-3.3	0.	0.	-4.2	0.0	1.4	-4.8	0.0	0.1	-3.3	0.
WL	-0.1	-3.3	0.	-1.4	-4.8	0.0	0.	-4.2	0.0	0.1	-3.3	0.
WRL+	0.	0.	0.	0.	-7.5	3.5	0.	-7.5	3.5	0.	0.	0.
WRL-	0.	0.	0.	0.	0.	0.2	0.	0.	0.2	0.	-5.6	6.9
E+	0.	0.	0.	0.	0.	-0.2	0.	0.	-0.2	0.	5.6	0.
E-	0.	0.	0.	0.	1.5	0.	1.8	-1.5	0.	0.	0.	0.
CL	0.	0.	0.	0.	6.4	0.5	0.	6.4	0.5	0.	0.	0.

LOAD GROUP DESCRIPTION	
D	: Dead load
C	: Collateral load
L	: Live load
W+	: Wind load as an inward acting pressure
W-	: Wind load as an outward acting suction
WR	: Wind force from the right
WL	: Wind force from the left
W16+	: Min. 16 psf wind as an inward acting pressure
E+	: Seismic force acting inward
E-	: Seismic force acting outward
ER	: Seismic force from right
EL	: Seismic force from left

NOTES

- 1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
- 2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
  - a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
  - b) RIGID FRAMES
  - c) GABLED BUILDINGS
    - (a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
    - (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
  - d) SINGLE SLOPE BUILDINGS
    - (a) LEFT COLUMN IS THE LOW SIDE COLUMN.
    - (b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
    - (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
  - e) ENDWALLS
    - (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
    - (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
  - f) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
  - g) ANCHOR RODS ARE ASTM F1554 GR. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
- 3) X-BRACING
  - (1) ROY BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN ON THE REACTION TABLES.
  - (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RUBPEU AND RBDWEU) DO NOT INCLUDE THE AMPLIFICATION FACTOR,  $R_d$ .
  - (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RUBPEU & RBDWEU) ARE MULTIPLIED BY FORCE REDUCTION FACTOR,  $R_d$ , WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO  $S_{ds}(0.2)$  IS GREATER THAN 0.45.
- 4) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
- 5) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2012 UBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH I VALUE WITH A LOAD FACTOR OF 1.0.
- 6) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH I LEVEL AND DO NOT CONTAIN THE  $R_h$  FACTOR.
- 7) FOR UBC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE  $R_d$  FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR FOUNDATION DESIGN. PROCEDURES AND ALLOW FOR AN ECONOMIC FOUNDATION DESIGN.

[illegible]

<p>Cornerstone Building Brands 13105 Northwest Freeway, Suite 500 Houston, TX 77040 cornerstonebuildingbrands.com</p>	<p><i>Project Name &amp; Location:</i> RAJULI MBORG 1529 N RIVER RD PASADENA, CA 93448-7325</p>	<p><i>Issued For Construction</i></p> <p><input type="checkbox"/></p>
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Scale:	NOT TO SCALE
Drawn by:	HPR 1/6/25
Checked by:	HPR 1/6/25
Project Engineer:	BXT
Job Number:	19-B-82019

**Sheet Number:** F3 of 3

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**BRENDAN PATRICK TOOHEY, P.E.**  
**CALIFORNIA P.E. C96062**

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 and Party Certificate Authority on any electronic copy.

*Brian*

REGISTERED PROFESSIONAL ENGINEER  
 BRENDAN PATRICK TOOMEY  
 C96062  
 JAN 06, 2025  
 CIVIL  
 STATE OF CALIFORNIA

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PERMIT DRAWINGS.



Builder/Contractor Responsibilities

**Drawing Validity**- These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

**Builder Acceptance of Drawings** Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice Sept 86 Section 4.2.1) (Mar 05 Section 4.4.1)

**Code Official Approval**- It is the responsibility of the Builder/Contractor to ensure that all project plans comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

**Building Erection** - The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work, or other elements required for erection, to be determined, furnished and installed by the erector (AISC Code of Standard Practice Sept 86 Section 7.9.1) (Mar 05 Section 7.10.3) (CSA/S16-09 Section 29).

**Discrepancies** - Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice Sept 86 Section 3.3) (Mar 05 Section 3.3)

**Materials by Others** - All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

**Modification of the Metal Building from Plans** - The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

**Foundation Design**- The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)

**Shimming** - In accordance with Section 6.10 of Chapter 4 Common Industry Practices in the Metal Building Systems Manual, shimming is a normal part of erection and is not subject to claim.

Building Descriptions			
Building ID	Width	Length	Height
Building A	68'-0"	60'-0"	18'-0"



Download panel installation manuals from:

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Sales: 800.879.7827  
starbuildings.com

DESIGN CRITERIA

Job Number ..... 19-B-82019  
Builder ..... DUECK CONSTRUCTION COMPANY INC-153691  
Jobsite Location ..... PAUL VIBORG, PASO ROBLES, CA  
Building Code ..... 2022 CALIFORNIA BUILDING CODE  
Building Risk Category ..... Normal (Risk Category II)  
Roof Dead Load  
Superimposed ..... 2.08 psf  
Collateral ..... 5.00 psf  
(5.00 psf Other)  
Roof Live Load ..... 20.00 psf reduction allowed  
Wind  
Ultimate Wind Speed (Vult) .. 95 mph  
Nominal Wind Speed (Vasd) ... 74 mph (IBC section 1609.3.1)  
Serviceability Wind Speed ... 64 mph  
Ground Elevation Factor ..... 0.98 (697 ft ASL)  
Wind Exposure Category ..... C  
Exposure Coefficient (MWFRS): 0.882  
Enclosure Classification .... Enclosed Building  
Internal Pressure Coef (GCp1): 0.18/-0.18  
Wall loads for components not provided by building manufacturer  
Zone 5 Areas (within 6.00' of corner) : 18.24 psf pressure -24.32 psf suction  
Zone 4 Areas (away from corners) : 18.24 psf pressure -19.76 psf suction  
These values are the maximum values required based on a 10 sq ft area.  
Components with larger areas may have lower wind loads.  
Seismic  
Seismic Importance Factor (Ie): 1.00  
Seismic Design Category ..... D  
Soil Site Class ..... D Stiff Soil (Default)  
Ss ..... 1.169 g Sds ..... 0.935 g  
Sl ..... 0.426 g Sd1 ..... 0.532 g  
Analysis Procedure ..... Equivalent Lateral Force  
Column Line ..... 4 1-3 SWA & SWC  
Basic Force Resisting System B3 C4 B3  
Response Modification Coefficient (R) 3.25 3.50 3.25  
Seismic Response Coefficient (Cs) 0.288 0.267 0.288  
Design Base Shear in kips (V) 1.63 8.77 11.78  
Basic Structural System (from ASCE 7-16 Table 12.2-1)  
B3 - Ordinary Steel Concentrically Braced Frame  
C4 - Ordinary Steel Moment Frame

DEFLECTION CRITERIA

Job Number ..... 19-B-82019  
Builder ..... DUECK CONSTRUCTION COMPANY INC-153691  
Jobsite Location ..... PAUL VIBORG, PASO ROBLES, CA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS ..... BLDG-A

Roof Limits	Rafters	Purlins	Panels
Live: L/	180	150	60
Serviceability Wind: L/	180	180	60
Total Gravity: L/	120	120	60
Total Uplift: L/	N/A	N/A	60
Frame Limits	Sidesway		
Live: H/	60		
Serviceability Wind: H/	60		
Seismic Drift: H/	40		
Total Gravity: H/	60		
Service Seismic: H/	40		
Wall Limits	Limit		
Total Wind Panels: L/	60		
Total Wind Girts: L/	90		
Total Wind EW Columns: L/	120		

The Service Seismic limit as shown here is at service level loads.

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Houston, TX 77040  
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Field Services: 844.840.4603  
field.services@cornerstone-bb.com

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield. For Canada, material properties conform to CAN/CSA G40.20/G40.21 or equivalent.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1, fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-136.

Bolted joints with A325 Type 1 bolts greater than 1/2" diameter are specified as pre-tensioned joints in accordance with the most recent edition of the RCSC Specification for Structural Joints Using ASTM A325 or A490 Bolts. Pre Tensioning can be accomplished by using the turn-of-nut method of tightening, calibrated wrench, twist-off-type tension-control bolts or direct-tension indicator as acceptable to the Inspecting Agency and Building Official. Installation inspection requirements for pre-tensioned joints (Specification for Structural Joints Section 9.2) using turn-of-nut method is suggested. The connections on this project are not slip critical.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the metal building manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

This project is designed using manufacturer's standard serviceability criteria. Generally this means that all deflections are within typical performance limits for normal occupancy and standard metal building products.

This metal building system is designed as an Enclosed Building. Exterior and/or operable components including, but not limited to, doors, windows, vents, etc. ("Components") must be designed to withstand the required component and cladding wind pressures specified by the building code. In order to maintain the metal building system's Enclosed Building condition, all Components shall be closed when wind velocities reach half the designed wind load for the metal building system as shown on the drawings and design criteria documentation. Failure to maintain the metal building system's Enclosed Building condition will violate and void all warranties and certifications applicable to the material supplied by the metal building manufacturer.

The materials by the manufacturer will be fabricated in a facility that has received Certification of Accreditation for the Manufacture of Metal Building Systems (AC472) from International Accreditation Service (IAS). This certification is recognized under Section 1704 of the IBC for approved fabricator.

The framing at building A, gridline 1 is NOT designed to receive a future bay addition. Corresponding frame reactions are calculated based upon actual tributary area.

Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

Using 8.1 x 6.3125 eave gutter with 4 x 5 downspouts, the roof drainage system has been designed using the method outlined in the MBMA Metal Building Systems Manual. Downspout locations have not been located on these drawings. The downspouts are to be placed on the building sidewalls at a spacing not to exceed 60 feet with the first downspout from both ends of the gutter run within 30 feet of the end. Downspout spacing that does not exceed the maximum spacing will be in compliance with the building code. The gutter and downspout system as provided by the manufacturer is designed to accommodate 2.9 in/hr rainfall intensity.

Drawing Index		Ck'd	By	Description	Date	Revision
Page	Description					
F1	Anchor Rod Setting Plan					
F2	Anchor Rod Details					
F3	Anchor Rod Reactions					
E1	Cover Sheet					
E2	Primary Steel Location Plan					
E3	Roof Framing Plan					
E4	Roof Sheeting Plan					
E5	Sidewall Elevation SWA					
E6	Sidewall Elevation SWC					
E7	Endwall Elevation EWB					
E8	Endwall Elevation EWD					
E9	Cross Section at Frame Line 1					
E10	Cross Section at Frame Line 2					
E11	Cross Section at Frame Line 3					
E12	Connection Detail					
R1-R3	Erection Guide					
R4-R10	Construction Drawings					

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13105 Northwest Freeway, Suite 500  
Houston, TX 77040  
cornerstonebuildingbrands.com

Project Name & Location:  
PAUL VIBORG  
1529 N RIVER RD  
PASO ROBLES, CA 93446-7325  
ATTN: TIM DUECK

Customer:  
DUECK CONSTRUCTION COMPANY INC  
2313 SIGNORA ROSA CT  
PASO ROBLES, CA 93446  
ATTN: TIM DUECK

Drawing Status: ☐ Issued For Approval ☒ Issued For Construction ☐ Issued For Permit

Scale: NOT TO SCALE

Drawn by: HPR 1/6/25

Checked by: HPR 1/6/25

Project Engineer: BXT

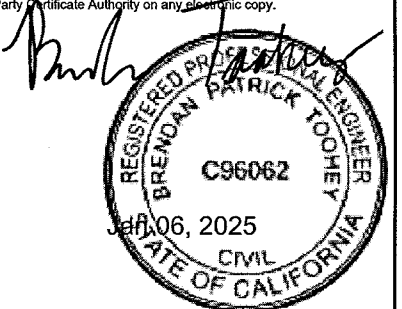
Job Number: 19-B-82019

Sheet Number: E1 of 12

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BRENDAN PATRICK TOOHEY, P.E.  
CALIFORNIA P.E. C96062

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SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
	0'-0 1/4"		2'-5 3/4"
	0'-3 3/4"		3'-1 3/4"
	1'-5 3/4"	REFER TO CF01122	

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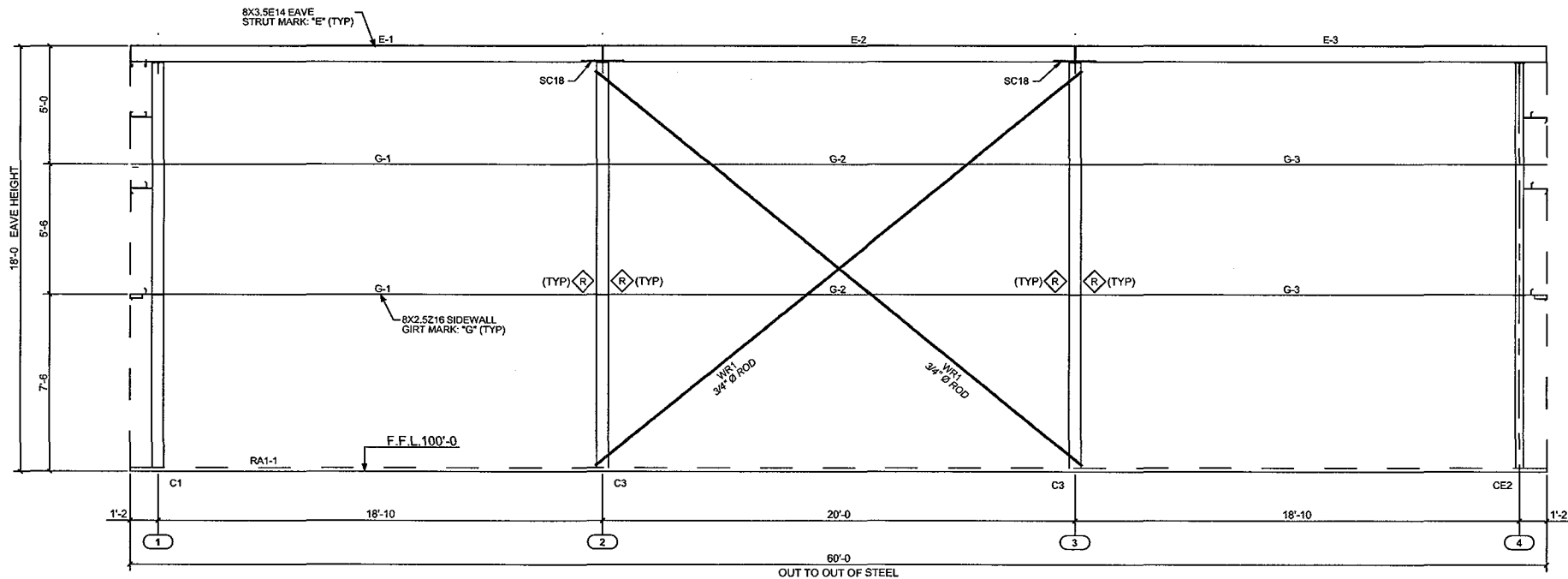
*Paul*

*Patrick Tooley*

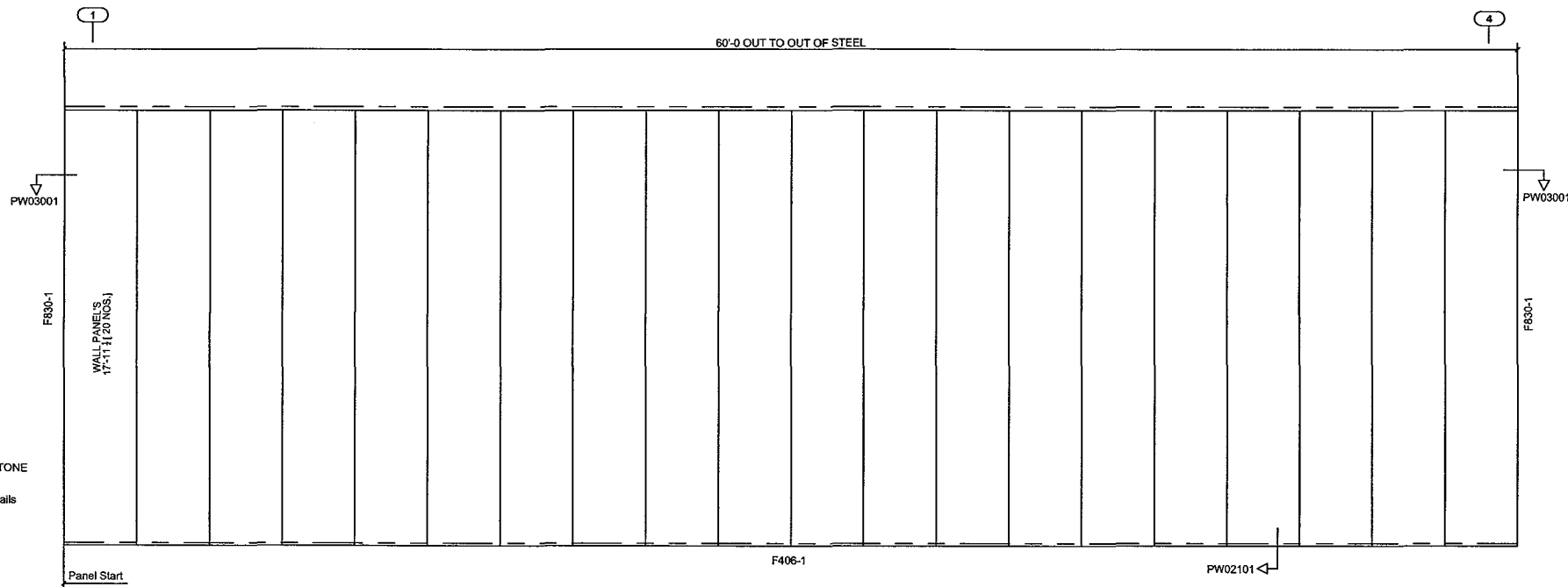
REGISTERED PROFESSIONAL ENGINEER  
BRENDAN PATRICK TOOLEY  
C96062  
JAN 06, 2025  
CIVIL  
STATE OF CALIFORNIA

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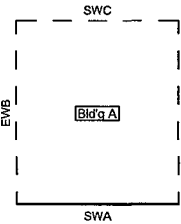


Sidewall Framing SWA at Grid Line A



Sidewall Sheeting SWA

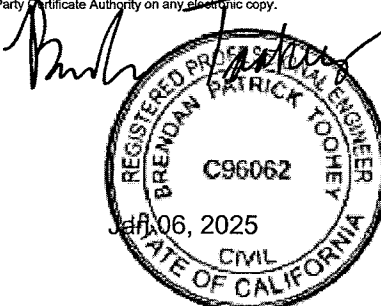
PBR Wall Panels  
Panel Coverage = 3'-0"  
Panel Color = S200 LIGHT STONE  
Panel Pkg. Req'd. = PBS-3  
Field Cut Panel and Trim as  
required per Construction Details



Key Plan

ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
	0'-0 1/4"		2'-5 3/4"
	0'-3 3/4"		3'-1 3/4"
	1'-5 3/4"	REFER TO CF01122	

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Revision	Date	Description	By	CK'd

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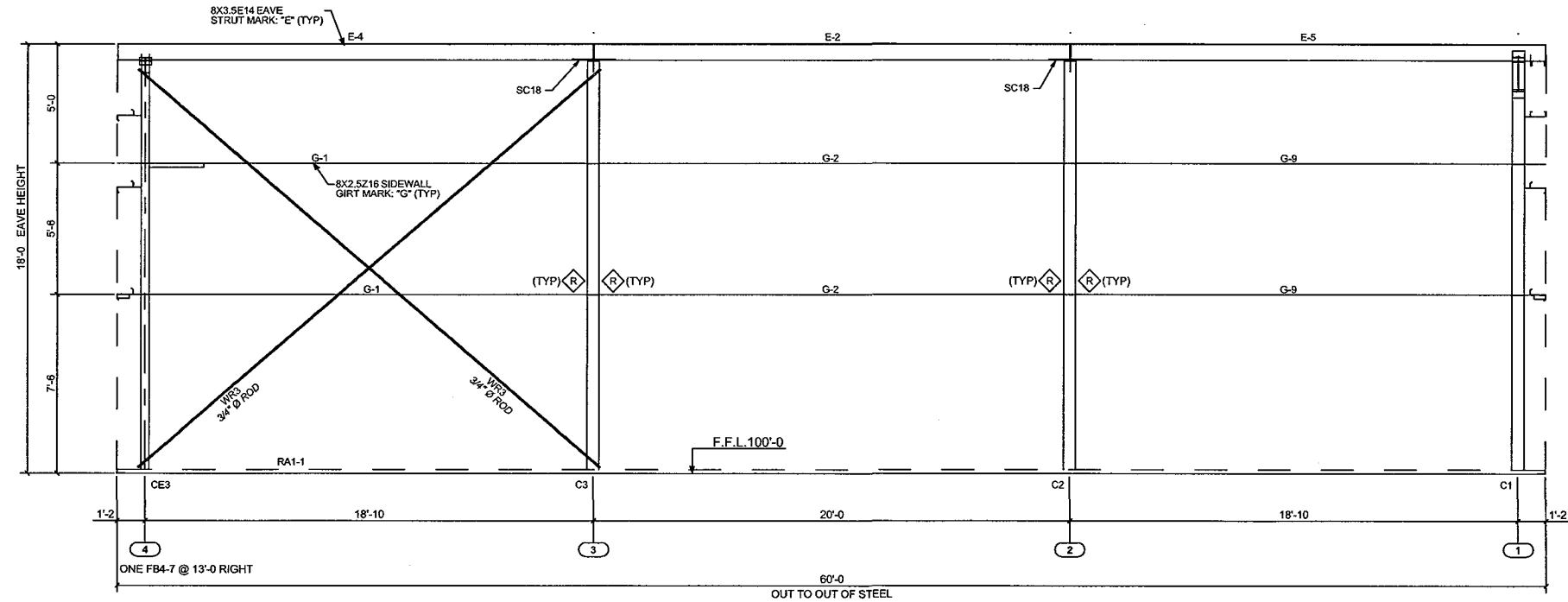
**Project Name & Location:**  
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1529 N RIVER RD  
PASO ROBLES, CA 93446-7325  
ATTN: TIM DUECK

**Customer:**  
DUECK CONSTRUCTION COMPANY INC  
2313 SIGNORA ROSA CT  
PASO ROBLES, CA 93446  
ATTN: TIM DUECK

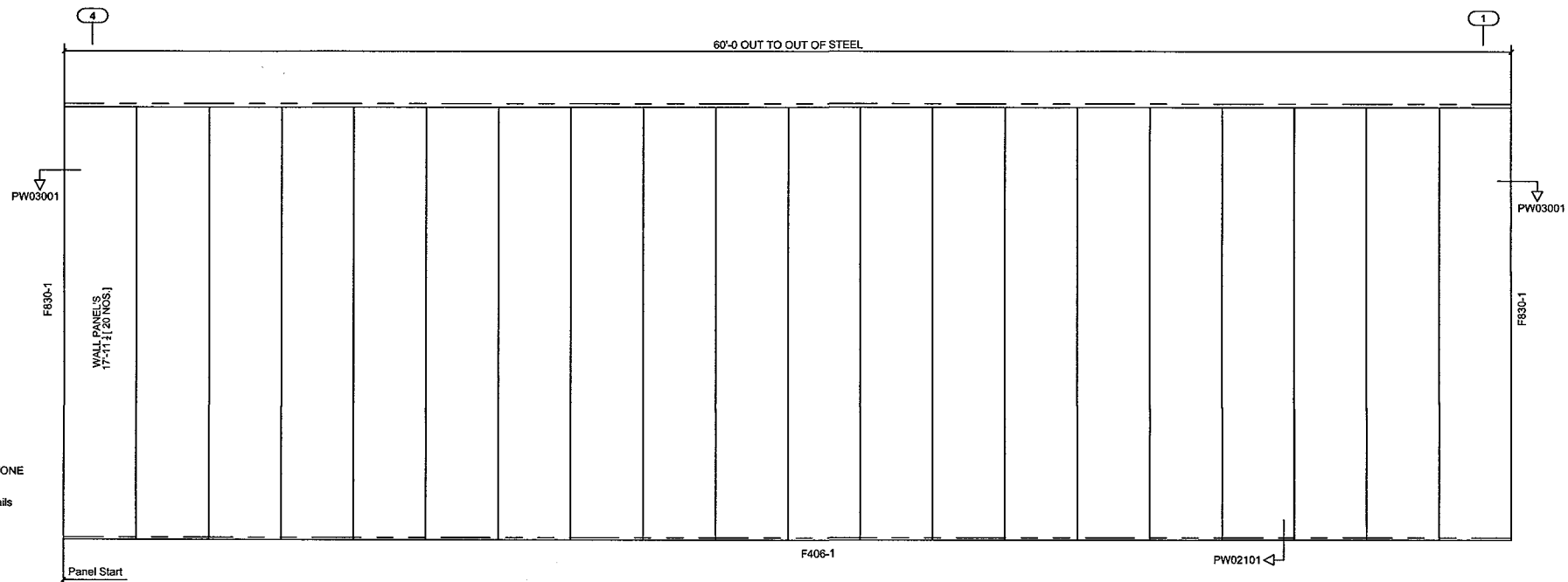
**Drawing Status:**  
☐ Issued For Approval  
☐ Issued For Construction  
☒ Issued For Permit

Scale: NOT TO SCALE  
Drawn by: HPR 1/6/25  
Checked by: HPR 1/6/25  
Project Engineer: BXT  
Job Number: 19-B-82019  
Sheet Number: E5 of 12  
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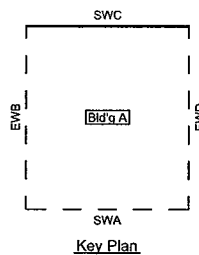


Sidewall Framing SWC at Grid Line D



### Sidewall Sheeting SWC

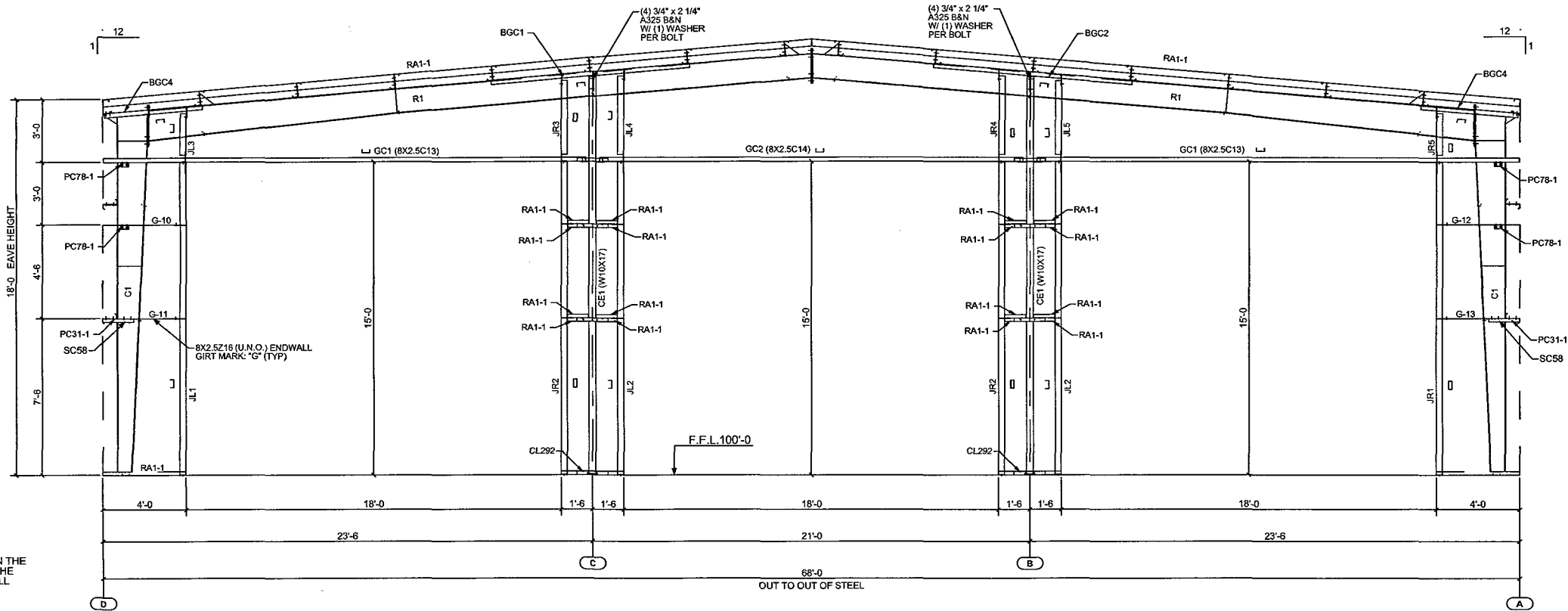
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	0'-3 3/4"		3'-1 3/4"
	1'-5 3/4"	REFER TO CF01122	



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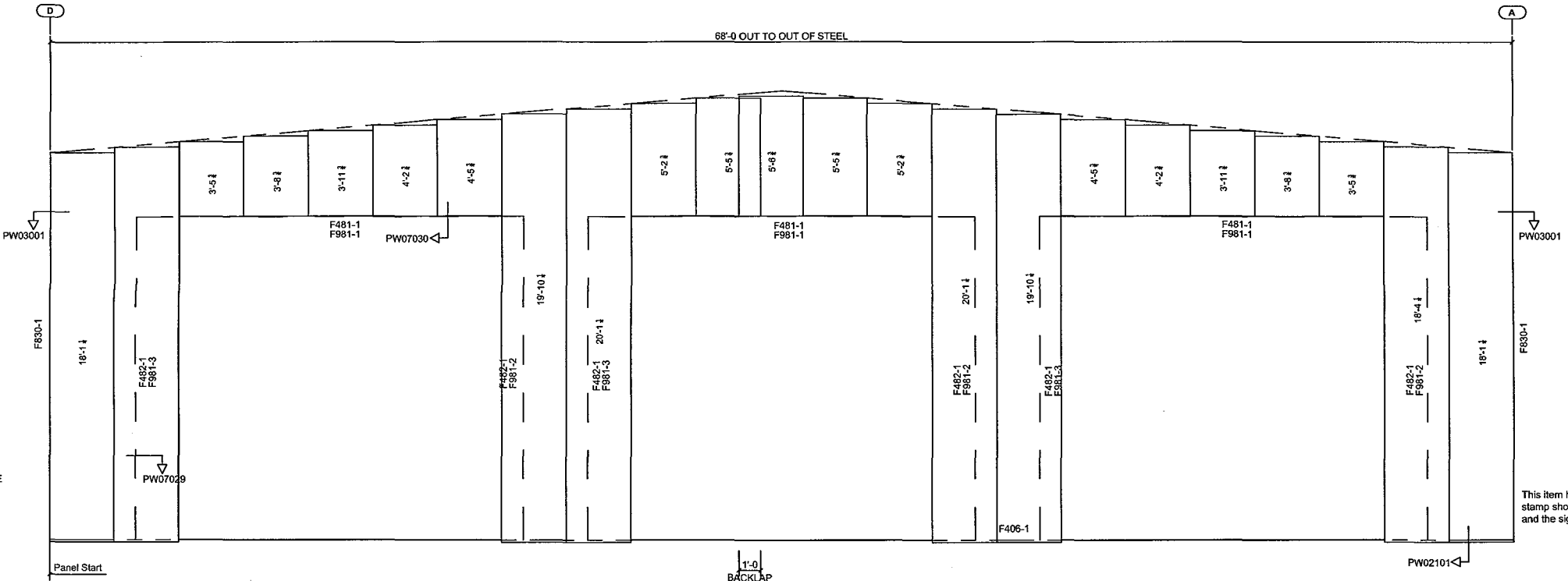
DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.  
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PERMIT DRAWINGS.

CL292- FASTENS BETWEEN THE GIRTS ON EACH SIDE OF THE ENDWALL COLUMNS, AT ALL GIRT ELEVATIONS. REFER TO DETAILS.

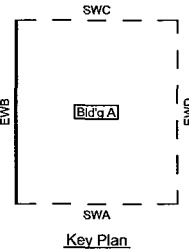


Endwall Framing EWB at Grid Line 1  
ALL JAMB AND SUB JAMB SIZE IS 8X3.5C14 TYP.U.N.O.

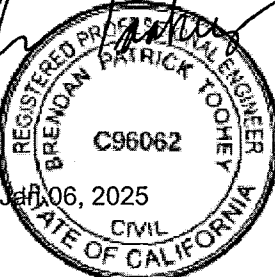
PBR Wall Panels  
Panel Coverage = 3'-0"  
Panel Color = S200 LIGHT STONE  
Panel Pkg. Req'd. = PBS-1  
Field Cut Panel and Trim as required per Construction Details



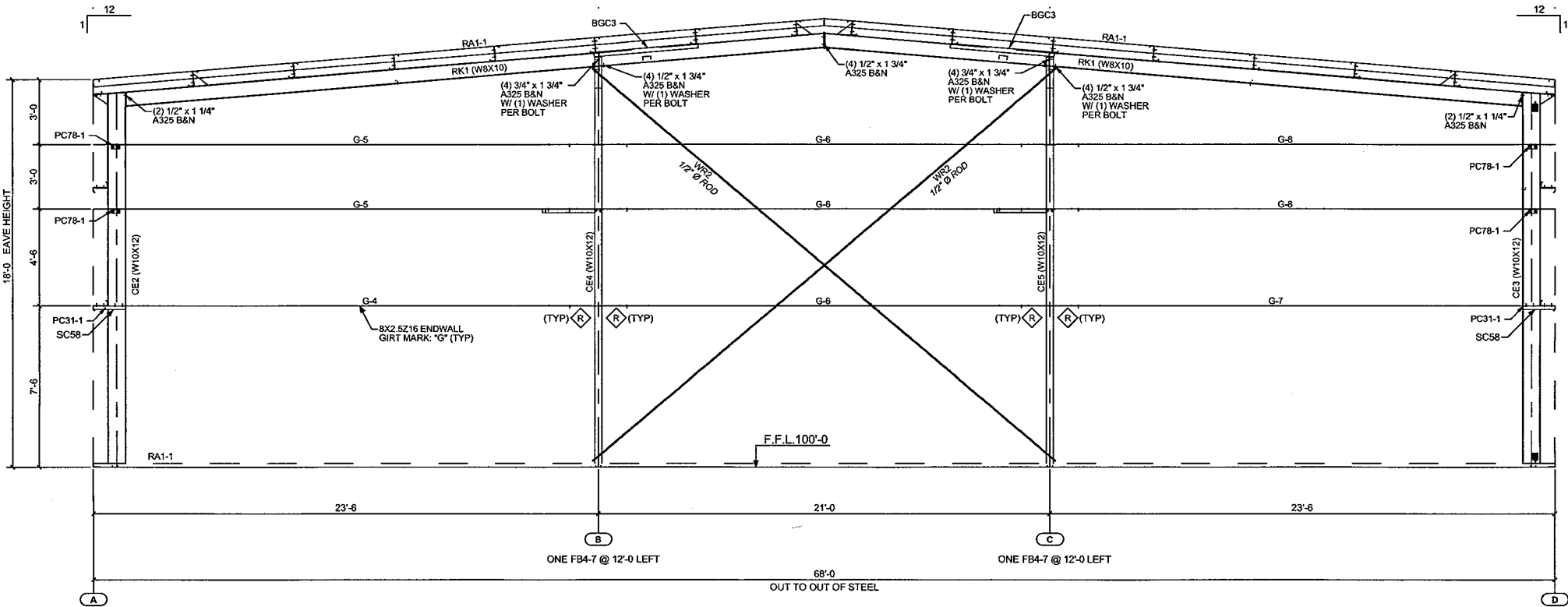
Endwall Sheeting EWB



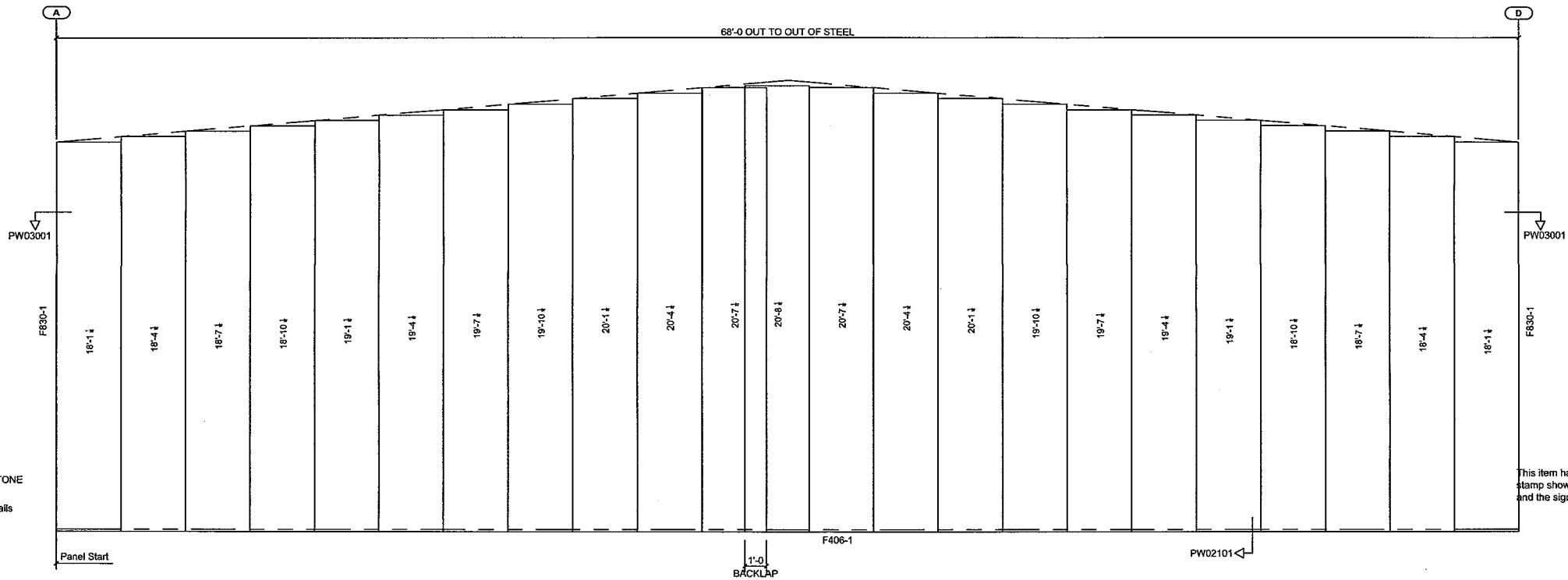
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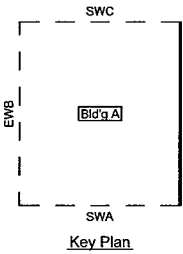
DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.  
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE PERMIT DRAWINGS.



Endwall Framing EWD at Grid Line 4



Endwall Sheeting EWD



PBR Wall Panels  
Panel Coverage = 3'-0"  
Panel Color = S200 LIGHT STONE  
Panel Pkg. Req'd. = PBR-2  
Field Cut Panel and Trim as  
required per Construction Details

ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
	0'-0 1/4"		2'-5 3/4"
	0'-3 3/4"		3'-1 3/4"
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By		Rev'd	
Description		Date	
Revision			
Cornerstone Building Brands 13105 Northwest Freeway, Suite 500 Houston, TX 77040 cornerstonebuildingbrands.com			
Project Name & Location: PAUL VIBORG 1529 N RIVER RD PASO ROBLES, CA 93446-7325 ATTN: TIM DUECK			
Customer: DUECK CONSTRUCTION COMPANY INC 2313 SIGNORA ROSA CT PASO ROBLES, CA 93446 ATTN: TIM DUECK			
Drawing Status: <input type="checkbox"/> Issued For Approval <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Permit			
Scale: NOT TO SCALE			
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Checked by: HPR 1/6/25			
Project Engineer: BXT			
Job Number: 19-B-82019			
Sheet Number: E8 of 12			
The engineer whose seal appears hereon is employed by or is contracted to provide engineering services for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.			
BRENDAN PATRICK TOOHEY, P.E. CALIFORNIA P.E. C96062			



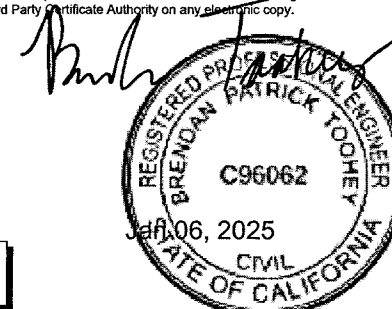
PRIMARY BUILT-UP MEMBER SIZES							
MARK	OUTSIDE FLG		INSIDE FLG		THICK	WEB	
	THICK	WIDTH	THICK	WIDTH		START DEPTH	END DEPTH
1	0.2500	6"	0.2500	6"	0.1340	8.0000	13.7351
2	0.2500	6"	0.2500	6"	0.1850	13.7351	16.9999
3	0.2500	6"	0.2500	6"	0.1340	17.0000	13.5000



GENERAL NOTE:  
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND  
MAY VARY DUE TO CONDITIONS (DEFLECTION).  
VERTICAL CLEARANCE DIMENSIONS ARE FROM  
FINISHED FLOOR REFERENCE ELEVATION.

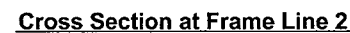
BRENDAN PATRICK TOOHEY, P.E.  
CALIFORNIA P.E. C96062

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DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.  
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE  
PERMIT DRAWINGS.

MARK	OUTSIDE FLG THICK		INSIDE FLG THICK		WEB		
	WIDTH	WIDTH	WIDTH	WIDTH	THICK	START DEPTH	END DEPTH
1	0.2500	6"	0.2500	6"	0.1850	8.0000	27.0000
2	0.2500	6"	0.3125	6"	0.1560	23.0000	11.5000
3	0.2500	6"	0.2500	6"	0.1340	11.5000	11.5000



GENERAL NOTE:  
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND  
MAY VARY DUE TO CONDITIONS (DEFLECTION).  
VERTICAL CLEARANCE DIMENSIONS ARE FROM  
FINISHED FLOOR REFERENCE ELEVATION.

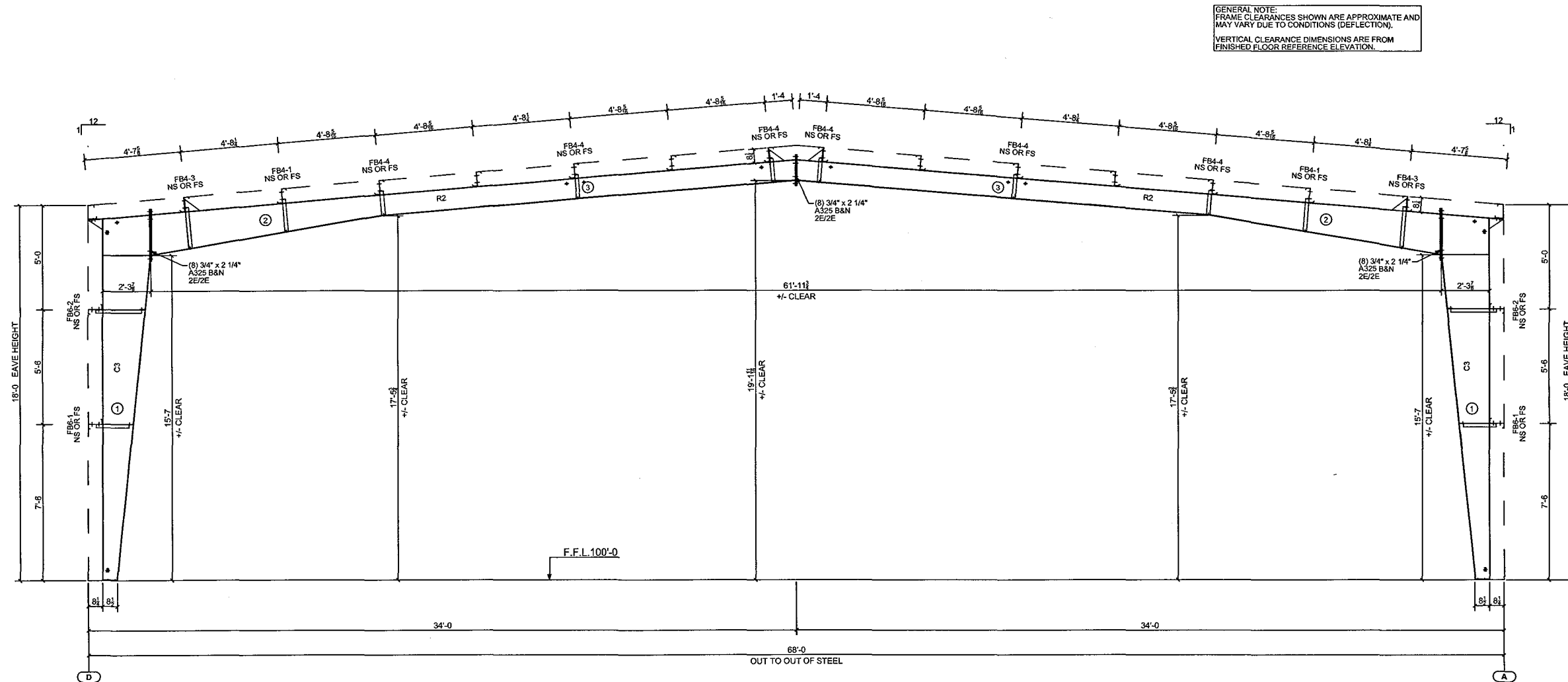
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## DRC Item 3

FRAME = Eng\19-B-82019\ver01-joshua.lorentson\BLDG-AIDrftg\01L (12/30/24 11:59:26)

APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
R2	602
C3	445



**Cross Section at Frame Line 3**

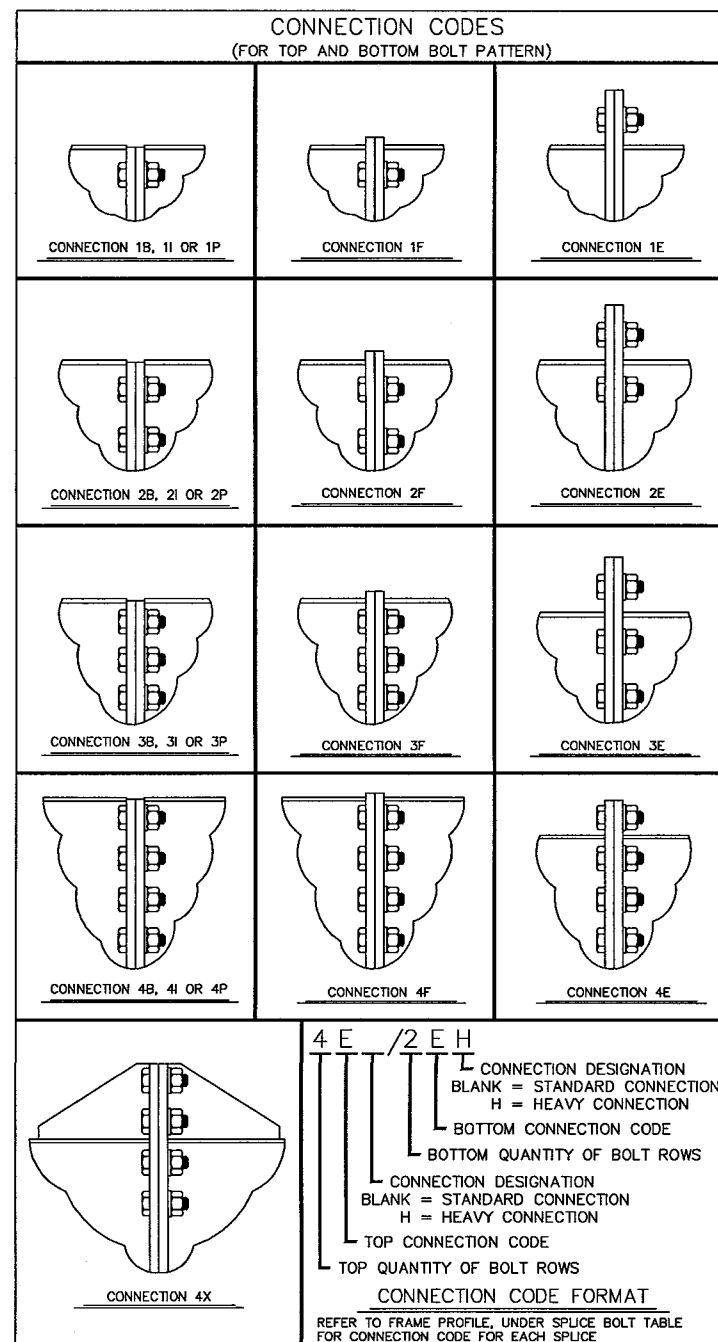
PRIMARY BUILT-UP MEMBER SIZES							
MARK	OUTSIDE FLG		INSIDE FLG		WEB		
	THICK	WIDTH	THICK	WIDTH	THICK	START DEPTH	END DEPTH
1	0.2500	6"	0.2500	6"	0.1850	8.0000	27.0000
2	0.2500	6"	0.3125	6"	0.1560	23.0000	11.5000
3	0.2500	6"	0.2500	6"	0.1340	11.5000	11.5000

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NAME	DESCRIPTION FOR A325 BOLT DIMENSIONS	A325 CONNECTION BOLT DIMENSIONS					
		1/2"	3/4"	7/8"	1"	1 1/4"	1 1/2"
D	DIAMETER OF THE BOLT	1/2"	3/4"	7/8"	1"	1 1/4"	1 1/2"
HD	BOLT HOLE DIAMETER	9/16"	13/16"	15/16"	1 1/16"	1 5/16"	1 9/16"
G	BOLT GAUGE	2 1/2"	3"	4"	3 1/2"	4"	5 1/2"
G	MAX. WEB THICKNESS (Max. 5/8" Fillet Weld) WITHOUT WASHER	1"	1 1/8"	1 7/8"	1 1/4"	1 3/8"	2 1/8"
G	MAX. WEB THICKNESS (Max. 5/8" Fillet Weld) WITH WASHER	3/4"	7/8"	1 5/8"	7/8"	7/8"	1 7/8"
HG	HEAVY CONN. BOLT GAUGE	N/A	2 1/4"	2 5/8"	3"	3 3/4"	4"
S	NORMAL BOLT SPACING	2 1/2"	3"	3 1/4"	3 1/2"	4"	4 1/2"
BSMIN	MINIMUM SPACING BETWEEN TOP & BOTTOM SETS OF BOLTS	1 1/2"	2 1/4"	2 5/8"	3"	3 3/4"	4"
BSMAX	MAXIMUM BOLT SPACING BETWEEN TOP AND BOTTOM SETS OF BOLTS ON CONNECTION PLATES LESS THAN OR EQUAL TO 3/4" THICK	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
BSMAX	MAXIMUM BOLT SPACING BETWEEN TOP AND BOTTOM SETS OF BOLTS ON CONNECTION PLATES LESS THAN OR EQUAL TO 3/4" THICK	SPLICE BOLT SPACING { 1/2 BSMAX (±1/8") WHEN BSMAX = 2'-0 1/8" TO 4'-0 (NOT TO EXCEED 2'-0) 1/3 BSMAX (±1/8") WHEN BSMAX = 4'-0 1/8" TO 6'-0 1/4 BSMAX (±1/8") WHEN BSMAX = 6'-0 1/8" TO 8'-0					
BFGD	MINIMUM BOLT-TO-FLANGE CLEARANCE AT OUT OF NUT SEE BOLT AT FLANGE DETAIL	1 1/2"	1 3/4"	1 7/8"	2 1/4"	2 1/2"	2 3/4"
PF	MINIMUM BOLT-TO-FLANGE CLEARANCE AT CONNECTION PLATE SEE BOLT AT FLANGE DETAIL	(BFGD + RNWT) PF INSIDE OF FLANGE IS INCREASED BASED ON THE YT & YB VALUE. PF FOR CONNECTION B, F, I AND P ARE THE SAME AS USED ON CONNECTION E					
NWT	NUT AND WASHER THICKNESS	SEE BOLT AT FLANGE DETAIL. NUT THICKNESS IS EQUAL TO THE BOLT DIAMETER AND .15625" WASHER THICKNESS IS USED EVEN IF A WASHER IS NOT REQUIRED.					
RNWT	RISE ON NUT AND WASHER THICKNESS	SEE BOLT AT FLANGE DETAIL. NUT THICKNESS IS EQUAL TO THE BOLT DIAMETER AND .15625" WASHER THICKNESS IS USED EVEN IF A WASHER IS NOT REQUIRED.					
TT	THICKNESS TOP FLANGE	REFER TO FRAME CROSS SECTION DRAWING FOR LARGEST FLANGE THICKNESS EITHER SIDE OF THE CONNECTION.					
TB	THICKNESS BOTTOM FLANGE	REFER TO FRAME CROSS SECTION DRAWING FOR LARGEST FLANGE THICKNESS EITHER SIDE OF THE CONNECTION.					
YT	BOLT SPACING TOP (ROUND UP TO NEXT 1/2", MIN = S)	3" + TT	3 1/2" + TT	3 3/4" + TT	4 1/2" + TT	5" + TT	5 1/2" + TT
YB	BOLT SPACING BOTTOM (ROUND UP TO NEXT 1/2", MIN = S)	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped
EED(E)	MINIMUM END EDGE DIMENSION	1 1/4"	1 1/4"	1 1/2"	1 3/4"	2 1/4"	2 5/8"
EED(S)	MINIMUM SIDE EDGE DIMENSION	3/4"	1"	1 1/8"	1 1/4"	1 5/8"	2 1/4"
EEDK	END EDGE DIMENSION AT KNEE CONNECTION	1 3/8"	1 3/8"	1 5/8"	1 7/8"	2 3/8"	2 3/4"
BCWM	MINIMUM BOLT CLEARANCE FROM A FLANGE OR WEB WELD	WITHOUT WASHER 7/16"	5/8"	3/4"	13/16"	1"	1 3/8"
BCWM	MINIMUM BOLT CLEARANCE FROM A FLANGE OR WEB WELD	WITH HARDENED WASHER 9/16"	3/4"	7/8"	1"	1 1/4"	1 1/2"
WCSM	MINIMUM WIDTH OF CONNECTION PLATE (Standard Connection)	5"	6"	8"	8"	10"	12"
WCHM	MINIMUM WIDTH OF CONNECTION PLATE (Heavy Connection)	N/A	10"	12"	12"	16"	18"
TCMIN	MINIMUM THICKNESS OF CONNECTION PLATE	1/4"	3/8"	7/16"	1/2"	5/8"	1"

Frame Documentation  
A325 Connection Bolt Details

05-12-10

Jun '18 04

B 4E/2EH

Connection Code  
(See "Connection Code Format"  
on this drawing)

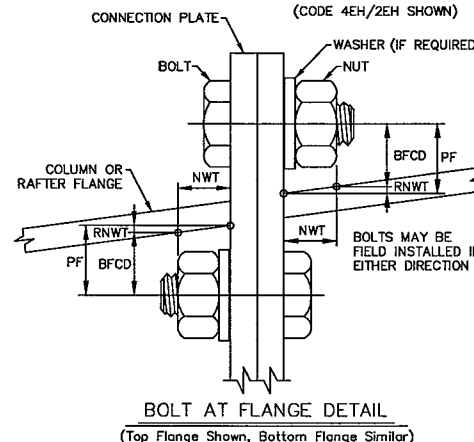
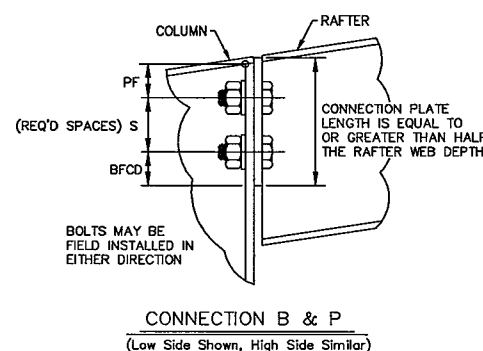
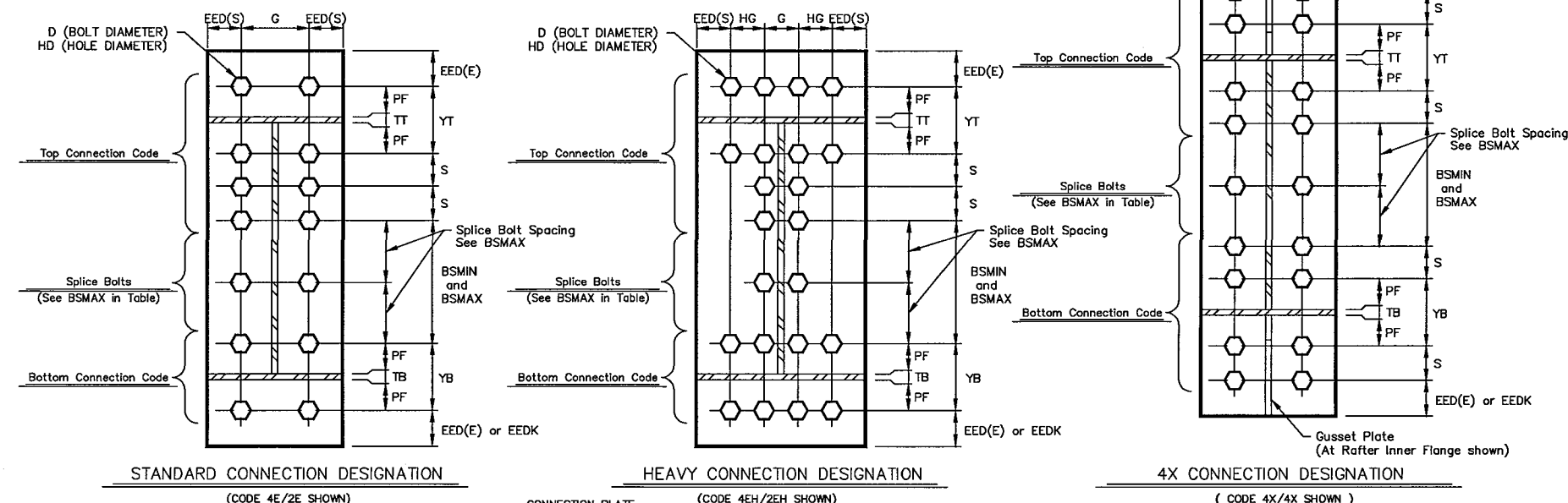
Connection Location

CROSS SECTION CONNECTION CODE KEY  
(AS SHOWN AT CONNECTIONS ON FRAME CROSS SECTION DRAWINGS)

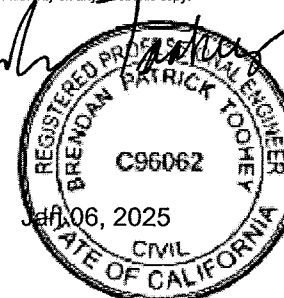
Flange Brace Material Schedule

Part Mark	Material
FB4	L 2" x 2" x 14 Ga.
FB5	L 2" x 2" x 14 Ga.
FB6	L 2" x 2" x 14 Ga.
FB7	L 2 1/2" x 2 1/2" x 16

CONNECTION BOLT DATA  
REFER TO 05-12-11 & 05-12-12 FOR BOLT LENGTH



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## DRC Item 4

### Project Request:

This is fence height modification request for the front yard fence located between house and property line fronting Olive Street. PRMC Section 21.44.030 requires front yard fences be no taller than 3-feet. A modification to allow a fence in the front yard to be up to 4-feet tall may be approved by the DRC.

